

India's First
Complete Guide to

Bar Bending Schedule

❑ Introduction:

Welcome to the comprehensive guide on Bar Bending Schedule (BBS) – an indispensable aspect of modern construction practices. This eBook is designed to unravel the intricacies of BBS, a fundamental process that bridges architectural designs with structural reality. Whether you're a budding engineer, a seasoned construction professional, or simply curious about the inner workings of building projects, this eBook will provide valuable insights into the world of reinforcement detailing and scheduling. Let's Start !!

❑ What You'll Learn:

1. **What You Will Learn:** In the following chapters, you will delve into the core principles of Bar Bending Schedule.
2. **Understanding BBS:** Gain a clear grasp of what Bar Bending Schedule entails, its significance, and how it influences the overall construction process.
3. **Essential Components:** Explore the crucial elements of BBS, including bar identification, types, sizes, cutting lengths, bending details, and accurate quantity calculations.
4. **Creating BBS:** Learn the step-by-step process of creating a meticulous BBS, from interpreting structural drawings to employing industry-standard practices.
5. **BBS Formats:** Discover various formats and templates used for organizing BBS data, enabling seamless communication between project stakeholders.
6. **Optimizing Efficiency:** Uncover strategies to optimize BBS to enhance project efficiency, minimize wastage, and ensure precise implementation.

❑ Conclusion:

As you conclude your journey through this eBook, you'll emerge equipped with a deep understanding of Bar Bending Schedule. Whether you're tasked with generating BBS documents, overseeing construction projects, or simply seeking to expand your knowledge, the insights gained from this eBook will undoubtedly serve you well. Let this eBook be your guiding light as you navigate the intricate realm of Bar Bending Schedule.

Note: To Understand Detailed Building Estimation, Enroll in Estimation & Costing eBook.

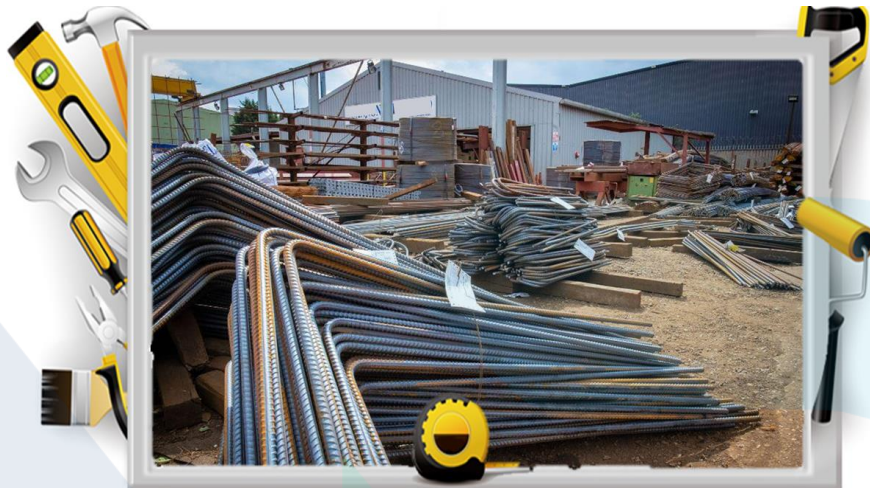
Sno.	Description	Page No.
01.	Introduction to Bar Bending Schedule	01 - 02
1.1	Bar Bending Schedule Sample Format	03
1.2	Advantages of Bar Bending Schedule	04
1.3	Uses of Bar Bending Schedule in Construction Project	05
1.4	Uses of Bar Bending Schedule in Construction Project	05
1.5	Why Steel Reinforcement is used in RCC Work	06
1.6	Different Grades of Steel Reinforcement	07
1.7	Different Diameters of Steel Reinforcement	08
1.8	Unit Weight of Steel Reinforcement	09
1.9	Details about Clear Cover & Effective Cover	10
1.10	Value of Clear Cover	11 - 12
1.11	Standard Codes Used in Bar Bending Schedule	13
1.12	Bend Deduction	14
02.	Symbols & Representation in Bar Bending Schedule	15 - 17
03.	Drawing Reading	18 - 21
04.	Drawing Reading	18 - 21
05.	Development Length & Lap Length	22 - 26
06.	Cutting Length Formula In Bar Bending Schedule	27 - 30
07.	IS Codes used in Bar Bending Schedule	31
7.1	IS Code Reference for Footing	32 - 33
7.2	IS Code Reference for Column	34 - 35
7.3	IS Code Reference for Beam	36 - 37
7.4	IS Code Reference for Slab	38

Sno.	Description	Page No.
08.	Bar Bending Schedule of RCC Component	39
8.1	Bar Bending Schedule of Stirrup	40 - 46
8.2	Bar Bending Schedule of Footing	47 - 49
8.3	Bar Bending Schedule of Column	50 - 52
8.4	Bar Bending Schedule of Beam	53 - 55
8.5	Bar Bending Schedule of Slab	56 - 60
8.6	Bar Bending Schedule of Circular Slab	61 - 65
09.	Bar Bending Schedule Automatic Excel Format	66 - 67
9.1	Bar Bending Schedule Automatic Format for Stirrup	68
9.2	Bar Bending Schedule Automatic Format for Footing	69
9.3	Bar Bending Schedule Automatic Format for Column	70
9.4	Bar Bending Schedule Automatic Format for Beam	71
9.5	Bar Bending Schedule Automatic Format for Slab	72
9.6	Bar Bending Schedule Atm. Format for Circular Slab	73
9.7	Bar Bending Schedule Atm. Format for Sunken Slab	74
10.	Bar Bending Schedule Sample Drawing	75 - 79

Introduction to BBS

Introducing Bar Bending Schedule

Bar bending, an essential aspect of building construction, involves the precise cutting and shaping of reinforcement bars. The process comprises several key steps that culminate in what is known as scheduling. In this book, we will cover the fundamental to advanced techniques of bar bending.



The first step in bar bending is the structural drawing reading which further includes:

- ❖ **Location and Marking:** Identifying the specific locations where bars are to be placed and marking them accordingly.
- ❖ **Type of Bar:** Specifying the type or grade of reinforcement bar to be used for different structural elements.
- ❖ **Size of Bar:** Determining the dimensions and diameter of each reinforcement bar.
- ❖ **Cutting Length:** Calculating the length to which each bar needs to be cut before bending and placing it.
- ❖ **Number of Bars:** Mentioning the quantity of bars required for each particular section of the structure.
- ❖ **Bending Details:** Providing precise instructions on how the bars should be bent to achieve the desired shape.
- ❖ **Total Quantity:** Summing up the total quantity of each type of reinforcement bar required as per the structural drawing.

All these detailed listings together form the essential process known as "Scheduling" in the context of Bar Bending.

Schedule Sheet Prepared In Excel

Bar Bending Schedule Sample Format

S.No.	Description	Shape of Bar	No. of Items	Dia of bar	No. of Bars	CL ₁ (m)	CL ₂ (m)	CL ₃ (m)	Cutting Length (m)	Total Length	Unit Wt.	Total Wt.	8mm (KG)	10mm (KG)	12mm (KG)	16 mm (kg)
FOOTING																
1	F1															
	X-dir	200  200	15	12 #	14				2.25	472.92	0.889	420.37	.	.	420.37	.
	Y-dir	200  200		12 #	10				1.75	262.80	0.889	233.60	.	.	233.60	.
2	F2															
	X-dir	200  200	8	12 #	12				1.75	168.19	0.889	149.50	.	.	149.50	.
	Y-dir	200  200		12 #	8				1.28	81.92	0.889	72.82	.	.	72.82	.

Please note that the format of the Bar Bending Schedule provided earlier is just one example among various formats utilized in construction companies. Each company may have its own specific processes and systems for creating Bar Bending Schedules. However, in the absence of a predefined company format, the mentioned format can serve as a suitable starting point for initiating the work.

It's essential to understand that Bar Bending Schedules are customized documents tailored to suit the unique requirements of each construction project. Companies may have specific templates and guidelines to follow, ensuring consistency and accuracy in the documentation process.

Before creating a Bar Bending Schedule, project teams should carefully analyze the structural drawings and specifications to determine the optimal reinforcement bar arrangement for each element.

Moreover, companies often maintain a library of standard bar shapes and bending templates, which can streamline the process and ensure adherence to quality standards.

Advantages of Bar Bending Schedule

- ❖ **Accuracy:** The schedule ensures precise cutting lengths and bending details for each reinforcement bar, reducing errors and wastage of materials.
- ❖ **Efficiency:** By having a detailed plan in advance, construction teams can work efficiently, saving time during the actual bar bending process.
- ❖ **Cost-Effectiveness:** The schedule helps in optimizing the use of reinforcement bars, leading to cost savings and better budget management.
- ❖ **Quality Control:** With a scheduled approach, the quality of bar bending work improves, leading to a stronger and more durable structure.
- ❖ **Coordination:** The schedule acts as a clear guideline for all involved parties, facilitating better coordination between engineers, contractors, and laborers.
- ❖ **Avoids On-Site Conflicts:** Having a pre-planned schedule reduces conflicts and confusions on-site, leading to a smoother construction process.
- ❖ **Structural Integrity:** Properly bent and positioned bars result in a structurally sound building that can withstand design loads and stresses.
- ❖ **Safety:** Accurate bar bending reduces the chances of accidents and mishaps during construction.
- ❖ **Documentation:** The schedule serves as a crucial piece of documentation for future reference, inspections, and maintenance.



- ❖ **Initial Planning:** During the initial planning stage, a preliminary BBS is not typically prepared. Instead, rough estimates of reinforcement quantities may be made based on high-level structural layouts. The BBS is not fully calculated at this stage, as it is more of a cost estimation exercise rather than a detailed engineering analysis. No specific drawing is associated with the BBS at this point.
- ❖ **Detailed Design:** In this stage, the BBS is calculated based on detailed structural drawings, such as the reinforcement layout plans, sections, and elevations. These drawings provide specific information about the structural elements' size, shape, and positioning. Engineers use this information to calculate the exact quantities, types, sizes, cutting lengths, and bending details of reinforcement bars required for each structural element.
- ❖ **Procurement:** The BBS calculated during the detailed design stage is used for procurement. The list of required reinforcement bars, along with their specifications, is used to order the materials from suppliers. The procurement team uses this information to ensure the right materials are purchased and delivered to the construction site.
- ❖ **Construction Execution:** During the construction phase, the BBS serves as a reference for the actual bar bending work. The site workers use the BBS to accurately cut, bend, and position the reinforcement bars. They follow the bending details provided in the BBS to ensure proper installation.
- ❖ **Revisions and Changes:** If design changes or unforeseen circumstances occur during construction that affect the reinforcement requirements, a revised BBS is prepared. This revised BBS incorporates the updated quantities, sizes, and bending details based on the changes made. The revised BBS is then used for further construction work.
- ❖ **Quality Assurance and Inspections:** Inspectors may refer to the BBS during quality assurance and inspections to ensure that the reinforcement bars are installed as per the design and construction specifications. They cross-check the actual work on-site against the details provided in the BBS.
- ❖ **As-Built Documentation:** After construction is complete, the actual positions and details of reinforcement bars are recorded as-built. This information is then updated in the BBS to create a final version that reflects the constructed reality. The as-built BBS serves as a valuable reference for future maintenance and renovations.

Why Steel Reinforcement is used in RCC work

- ❖ **Strength:** Steel reinforcement provides tensile strength to the concrete, making the composite material (RCC) capable of withstanding both compressive and tensile forces.
- ❖ **Coefficient of Thermal Expansion:** Coefficient of Thermal Expansion characteristics of reinforcement is almost similar to concrete.
- ❖ **Flexibility:** The use of steel reinforcement allows for flexibility in design, enabling construction of structures with various shapes, sizes, and spans.
- ❖ **Crack Control:** Steel bars help control and minimize cracks that may occur in concrete due to temperature changes, shrinkage, or loading.
- ❖ **Durability:** Steel reinforcement enhances the durability and longevity of RCC structures, as it mitigates the impact of environmental factors and loadings.
- ❖ **Load-Bearing Capacity:** The addition of steel bars significantly increases the load-bearing capacity of the RCC elements, allowing for the construction of taller and larger structures.
- ❖ **Ductility:** Steel is a ductile material, which means it can deform without breaking. This property allows RCC structures to absorb energy during earthquakes or other dynamic loads, making them more resistant to damage.
- ❖ **Fire Resistance:** Steel reinforcement provides some fire resistance to the concrete, as it acts as a heat sink, protecting the inner core from rapid temperature rise.
- ❖ **Construction Speed:** Using steel reinforcement speeds up construction since pre-cut bars can be easily placed and secured in the desired positions.
- ❖ **Cost-Effectiveness:** The use of steel reinforcement optimizes the use of materials, reducing overall construction costs while maintaining structural integrity.
- ❖ **Code Compliance:** Steel reinforcement in RCC work is essential for complying with building codes and design standards that require adequate reinforcement for safety and stability.

In construction, various grades of steel reinforcement are manufactured to meet specific engineering requirements. The most commonly used grades are:

- ❖ **Mild Steel (Grade I):** This is the most basic and commonly used grade of steel reinforcement, suitable for general construction purposes. It has a yield strength of 250 N/mm^2 .
- ❖ **Medium Tensile Steel (Grade II):** This grade offers higher strength than mild steel, with a yield strength of 415 N/mm^2 . It is used for structures that require increased load-bearing capacity.
- ❖ **High Strength Deformed Bars (HYSD) (Grade III and IV):** These bars have higher yield strength (500 N/mm^2 and 550 N/mm^2 , respectively) due to the presence of carbon, manganese, and other alloys. They are used in heavy and critical structures like bridges and high-rise buildings.
- ❖ **TMT Bars (Thermo-Mechanically Treated) (Fe 415, Fe 500, Fe 550, Fe 600):** TMT bars are manufactured with a special process that imparts superior strength and ductility. They are categorized based on their minimum yield strength, with Fe 415 having a yield strength of 415 N/mm^2 , Fe 500 having 500 N/mm^2 , Fe 550 having 550 N/mm^2 , and Fe 600 having 600 N/mm^2 . TMT bars are widely used in modern construction due to their excellent properties and cost-effectiveness.

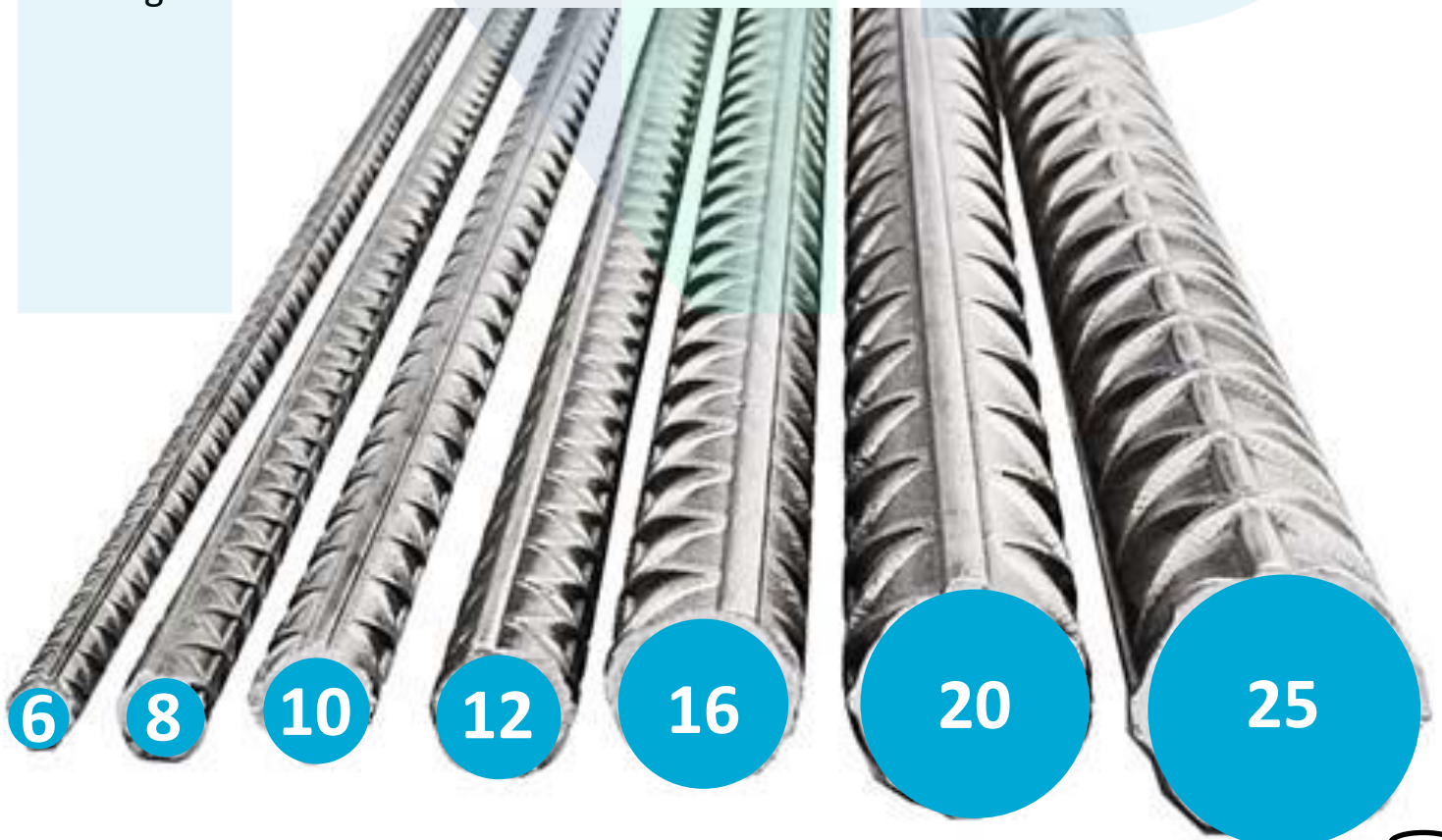
Each grade of steel reinforcement has its distinct advantages, and the selection of the appropriate grade depends on the structural requirements and design considerations of the specific construction project.



In the construction industry, there are various standard diameters of bars used for reinforcement purposes. The most commonly used diameters of bars are:

- ❖ 6 mm
- ❖ 8 mm
- ❖ 10 mm
- ❖ 12 mm
- ❖ 16 mm
- ❖ 20 mm
- ❖ 25 mm
- ❖ 32 mm
- ❖ 40 mm
- ❖ 50 mm

These diameters represent the cross-sectional size of the bars and are available in different lengths, typically ranging from 6 meters to 12 meters or more but generally 12 meters is used, depending on the supplier and project requirements. The selection of the appropriate bar diameter is based on the structural design, the load-carrying capacity required, and the specific construction needs. Larger diameter bars are generally used in heavy and critical structures, while smaller diameter bars are used in lighter and less critical elements.



Standard formula used for calculating unit weight of steel reinforcement:

$$\frac{D^2}{162}$$

Let's derive this standard formula of unit weight.

❖ = Unit Weight of Bar : Volume x Density

❖ = (Area x Length) x Density

❖ = $[(\pi/4) \times D^2 \times 1 \text{ m}] \times 7850 \text{ kg/m}^3$

❖ = $[(\pi/4) \times D^2 \times 1000] \times 7850 \frac{\text{kg}}{\text{mm}^3}$
1000mm x 1000mm x 1000mm

❖ = $D^2 \times 0.00616225$

❖ = $\frac{D^2}{1 / 0.00616225}$

❖ = $(D^2/162)$

Unit Conversion 'm' to 'mm & mm³'
(1m = 1000 mm
1m³ = 1000mm x 1000mm x 1000mm)

1m



Unit weight of different diameter of Bar.

❖ 08mm = $(8 \times 8)/162 = 0.395 \text{ kg/m}$

❖ 10mm = $(10 \times 10)/162 = 0.617 \text{ kg/m}$

❖ 12mm = $(12 \times 12)/162 = 0.889 \text{ kg/m}$

❖ 16mm = $(16 \times 16)/162 = 1.580 \text{ kg/m}$

❖ 20mm = $(20 \times 20)/162 = 2.470 \text{ kg/m}$

❖ 25mm = $(25 \times 25)/162 = 3.858 \text{ kg/m}$

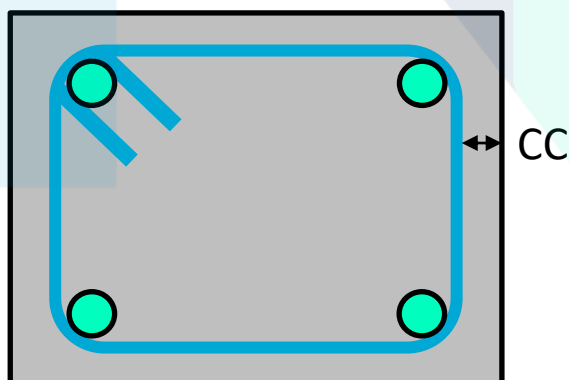
❖ 32mm = $(32 \times 32)/162 = 6.320 \text{ kg/m}$

Clear Cover:

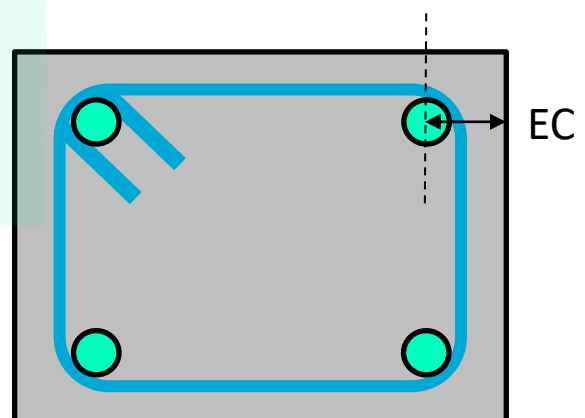
- ❖ Clear cover refers to the distance between the outer surface of the concrete and the nearest surface of the reinforcement bar.
- ❖ It is designed to provide protection to the reinforcement from environmental factors like moisture, chemicals, and carbonation, as well as to prevent corrosion.
- ❖ Clear cover also helps in maintaining the fire resistance of the concrete element.
- ❖ It is specified in building codes and standards to ensure the durability and safety of the structure.

Effective Cover:

- ❖ Effective cover refers to the distance between the center of the reinforcement bar and the nearest outer surface of the concrete.
- ❖ It takes into account the bar diameter and the concrete cover to determine the actual distance from the bar center to the concrete surface.
- ❖ Effective cover is crucial in structural design calculations as it affects the development length and anchorage of reinforcement.



CLEAR COVER



EFFECTIVE COVER

In Bar Bending Schedule (BBS), the clear cover is considered for detailing and scheduling purposes. The clear cover is essential in determining the bending and cutting length of the bars and ensuring they are placed at the correct position within the concrete element.

The values of clear cover used in different structural members can vary depending on factors like environmental conditions, structural design requirements, and building codes. Here are some general guidelines for clear cover values typically used in different structural elements:



1. Beams and Slabs:

- ❖ For normal environmental exposure and non-aggressive conditions: 20 mm to 25 mm.
- ❖ For moderate environmental exposure and mildly aggressive conditions: 30 mm to 40 mm.
- ❖ For severe environmental exposure and highly aggressive conditions: 50 mm or more.

2. Columns:

- ❖ For normal environmental exposure and non-aggressive conditions: 40 mm.
- ❖ For moderate environmental exposure and mildly aggressive conditions: 50 mm to 60 mm.
- ❖ For severe environmental exposure and highly aggressive conditions: 75 mm or more.

3. Footings and Pile Caps:

- ❖ For normal environmental exposure and non-aggressive conditions: 40 mm to 50 mm.
- ❖ For moderate environmental exposure and mildly aggressive conditions: 50 mm to 75 mm.
- ❖ For severe environmental exposure and highly aggressive conditions: 75 mm or more.

4. Walls:

- ❖ For normal environmental exposure and non-aggressive conditions: 25 mm to 30 mm.
- ❖ For moderate environmental exposure and mildly aggressive conditions: 30 mm to 40 mm.
- ❖ For severe environmental exposure and highly aggressive conditions: 40 mm to 50 mm.

5. Retaining Walls:

- ❖ For normal environmental exposure and non-aggressive conditions: 30 mm to 40 mm.
- ❖ For moderate environmental exposure and mildly aggressive conditions: 40 mm to 50 mm.
- ❖ For severe environmental exposure and highly aggressive conditions: 50 mm to 75 mm.

6. Staircases:

- ❖ For normal environmental exposure and non-aggressive conditions: 15 mm to 20 mm.
- ❖ For moderate environmental exposure and mildly aggressive conditions: 20 mm to 25 mm.
- ❖ For severe environmental exposure and highly aggressive conditions: 25 mm to 30 mm.

7. Slab-on-Grade (Ground Slabs):

- ❖ For normal environmental exposure and non-aggressive conditions: 20 mm to 25 mm.
- ❖ For moderate environmental exposure and mildly aggressive conditions: 30 mm to 40 mm.
- ❖ For severe environmental exposure and highly aggressive conditions: 50 mm or more.

8. Water Tanks (Reinforced Concrete):

- ❖ For normal environmental exposure and non-aggressive conditions: 25 mm to 30 mm.
- ❖ For moderate environmental exposure and mildly aggressive conditions: 30 mm to 40 mm.
- ❖ For severe environmental exposure and highly aggressive conditions: 40 mm to 50 mm.

- ☐ Please note that these values are approximate and may differ based on specific project requirements and local building codes. Designers and engineers should always refer to the applicable building standards and consult with structural consultants to determine the appropriate clear cover values for each project. Additionally, seismic considerations and fire resistance requirements may also influence the selection of clear cover values for specific structural elements.

Here are the names of some standard codes commonly used in Bar Bending Schedule (BBS) in :

India:

- ❖ IS 2502: Code of Practice for Bending and Fixing of Bars for Concrete Reinforcement (BBS-related guidelines).
- ❖ IS 1786: Specification for High Strength Deformed Steel Bars and Wires for Concrete Reinforcement (specifies the properties and grades of reinforcement bars).

United States:

- ❖ ACI 318: Building Code Requirements for Structural Concrete (provides guidelines for concrete design and reinforcement detailing).
- ❖ CRSI Manual of Standard Practice: Published by the Concrete Reinforcing Steel Institute, it covers standard practices for the industry.

United Kingdom (British):

- ❖ BS 8666: Scheduling, Dimensioning, Bending, and Cutting of Steel Reinforcement for Concrete (covers BBS-related guidelines).
- ❖ BS 4466: Specification for Higher Strength Cold Worked Steel Bars for the Reinforcement of Concrete (specifies properties and grades of reinforcement bars).

Saudi Arabia:

- ❖ SASO 42: Saudi Building Code (provides guidelines for structural design and reinforcement detailing in Saudi Arabia).

These standard codes play a crucial role in ensuring consistency, safety, and quality in the construction industry. They provide essential guidelines and specifications for the design, detailing, and construction of reinforced concrete structures, including the preparation of Bar Bending Schedules. Engineers, architects, and construction professionals refer to these codes to ensure that their designs and construction practices adhere to recognized industry standards.

❖ Bend Deduction in Bar Bending Schedule (BBS):

Bend Deduction refers to the length deducted from the total length of a reinforcement bar due to bending, so that the resulting bar length after bending corresponds to the required design length.

It is essential to account for bend deduction accurately in the BBS to ensure the reinforcement bars fit perfectly within the structural elements without affecting their strength and performance.

Why Bend Deduction is Done:

Bend Deduction is done to account for the elongation of the reinforcement bar during bending. When a bar is bent to form hooks, bends, or other shapes, its length increases due to bending radii and curvatures. To maintain the required design length, the bend deduction compensates for this elongation.

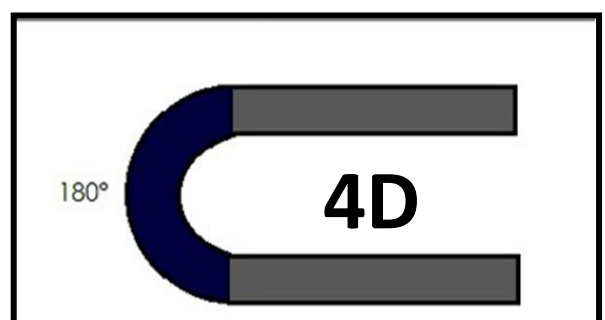
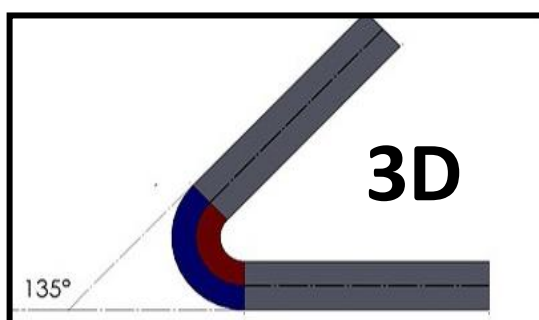
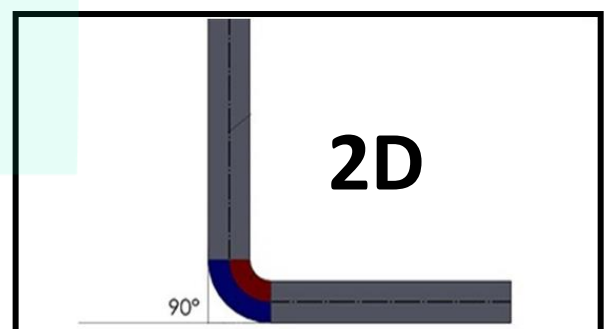
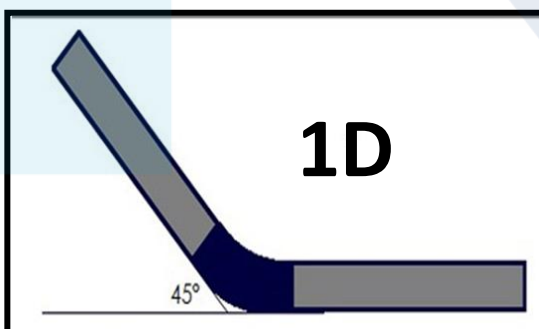
Types of Bend Deduction:

❖ Values of Bend Deduction:

The values of bend deduction vary depending on the specific bending angles, bending radii, and bar diameters used in the construction project.

They can be obtained from relevant design codes, industry standards, or bending machines' manufacturer's recommendations.

For example, 45-degree: 1D, 90-degree: 2D, 135-degree: 3D and 180-degree: 4D.



Symbols & Representation

In a Bar Bending Schedule (BBS), various symbols and representations are used to communicate essential information about the reinforcement bars and their arrangement in a clear and concise manner. These symbols help to identify different types of bars, bending details, and other relevant data. Here are some common symbols and representations used in BBS:

Bar Shape Symbols:

- ❖ Straight Bar: A straight line represents a straight bar without any bends.
- ❖ Bent Bar: A zigzag line or series of angles represent bent bars with specific bending details.

Bar Diameter and Spacing:

- ❖ The diameter of the bar is mentioned alongside the bar shape symbol, e.g., 12 Φ (12mm diameter bar).
- ❖ The spacing between parallel bars is indicated, e.g., @150 c/c (150mm center-to-center spacing).

Bend Detail Symbols:

- ❖ Right Angle Bend: A square or L-shaped symbol denotes a right-angle bend.
- ❖ U-shaped Bend: A rounded U-shaped symbol represents a U-bend.
- ❖ Hook Bend: A small semicircular symbol denotes a standard hook bend.
- ❖ Crank Bend: A symbol with a series of angles indicates a crank bend.
- ❖ Stirrup or Ring: A closed circular symbol represents a stirrup or ring.

BBS Header and Title:

The BBS contains a header with details like project name, drawing number, scale, date, and other project-specific information.

A clear title describes the purpose of the BBS and the specific structural element it covers.

Dimensional Units:

- ❖ The units of measurement used for bar dimensions, such as millimeters (mm) or centimeters (cm), are mentioned.

The BBS refers to the relevant reinforcement drawings or sections for further details and clarification.

These symbols and representations, when used consistently and correctly, make the Bar Bending Schedule a valuable document that facilitates efficient communication, accurate reinforcement fabrication, and precise construction of the reinforced concrete structure.

❑ Some Basic Symbol given Indian Standard 5525:1969

<i>Symbols Relating to Cross Sectional Shape and Size of Reinforcement Bars</i>	
	Plain round bar or Diameter of plain round bar
	Plain square bar or Side of plain square bar
	Deformed bar (including square twisted bar) or Nominal size (equivalent diameter or side) of the deformed bar (see Note)

Img.1.

<i>Symbols Relating to Various Structural Members</i>	
Bm	Beam(s)
Col	Column(s)
FG	Footing(s)
GR	Girder(s)
JT	Joist(s)
LL	Lintel(s)
LB	Lintel Beam(s)
Sb	Slab(s)
WL	Longitudinal Wall
WX	Cross Wall

Img.2.

<i>Symbols Relating to Position and Direction</i>	
EW	Each way
	Spacing centre to centre
	Limit of area covered by bars
	Direction in which bars extend

Img.3.

<i>Symbols Relating to Shape of the Bar Along Its Length</i>	
Bt	Bent bar
St	Straight bar
Stp	Stirrup
Sp	Spiral
Ct	Column tie

Img.4.

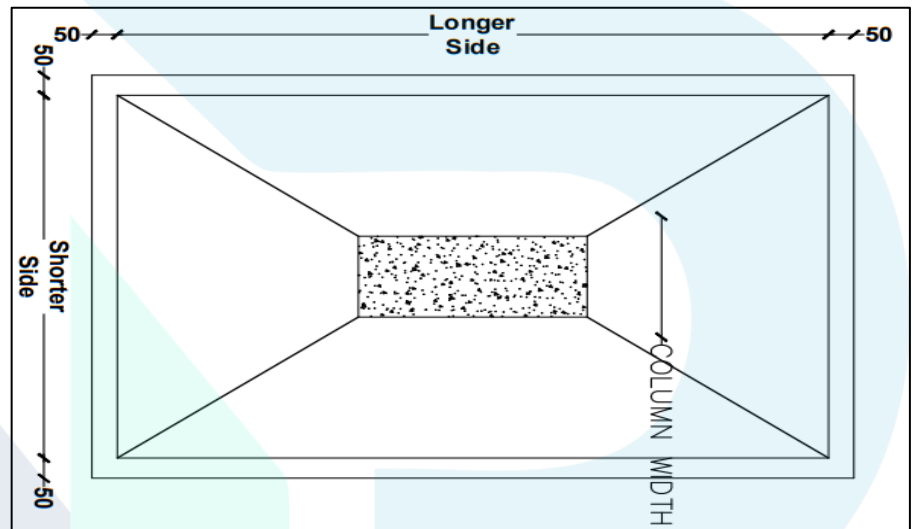
Drawing Reading

❖ **Footing Drawing:**

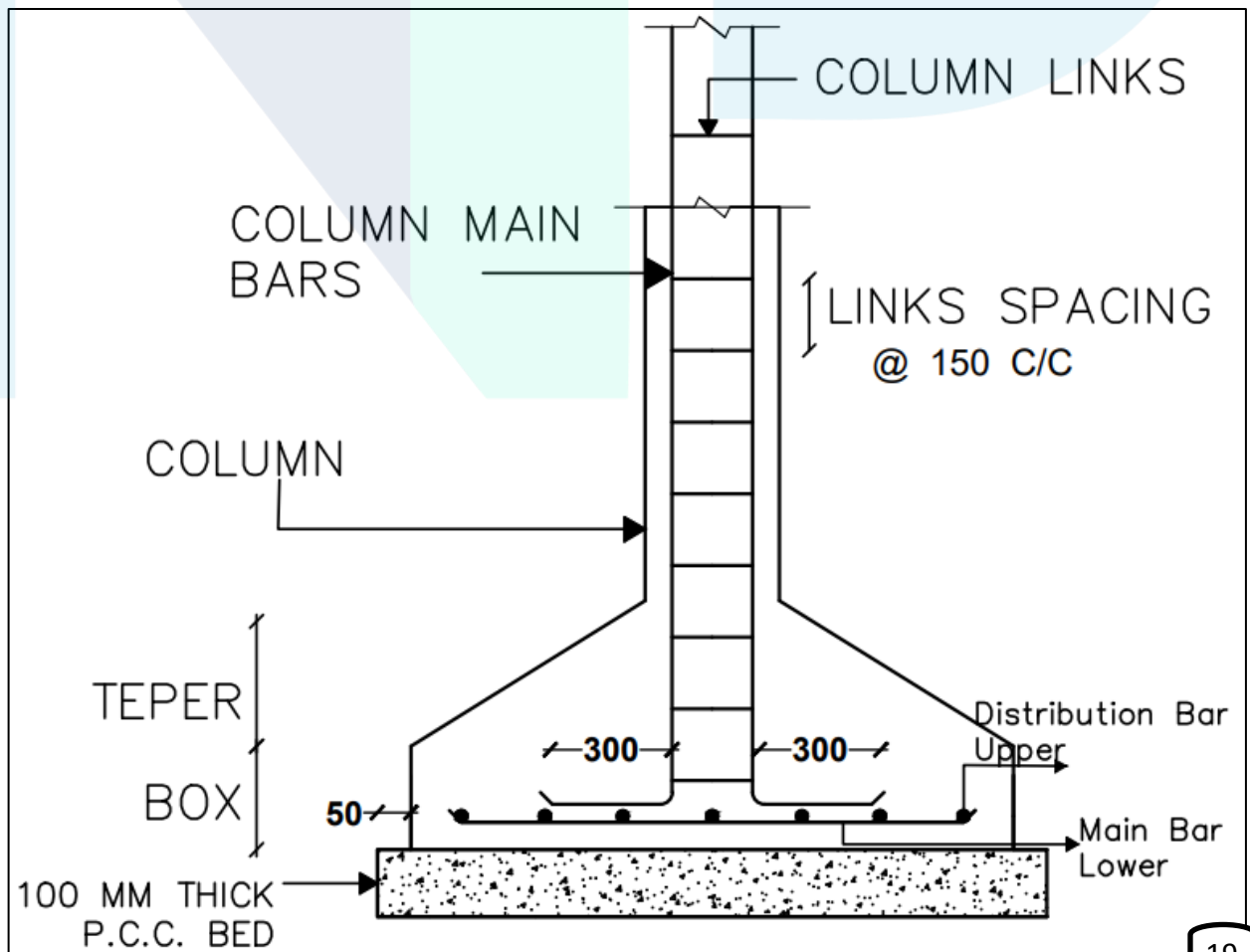
➤ Footing & Reinforcement Details

Dis.	EXCAVATION SIZE (m)	P.C.C THICK (m)	FOOTING SIZE (m)	FOOTING DEPTH		REINFORCEMENT ALONG	
				AT COLUMN FACE (m)	AT FOOTING END (m)	LONG LOWER TIER	SHORT UPPER TIER
F1	1x1.2	0.1	0.9x1.1	0.38	0.15	12 Φ	12 Φ

➤ Footing Plan

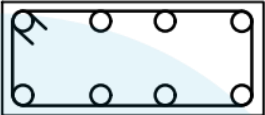


➤ Footing Section

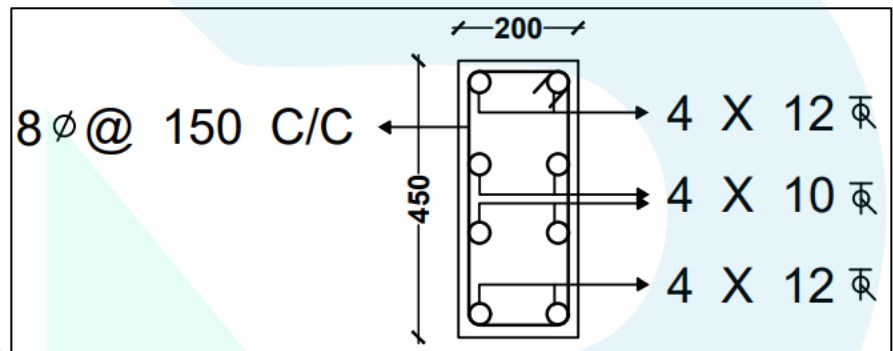


❖ Column Drawing:

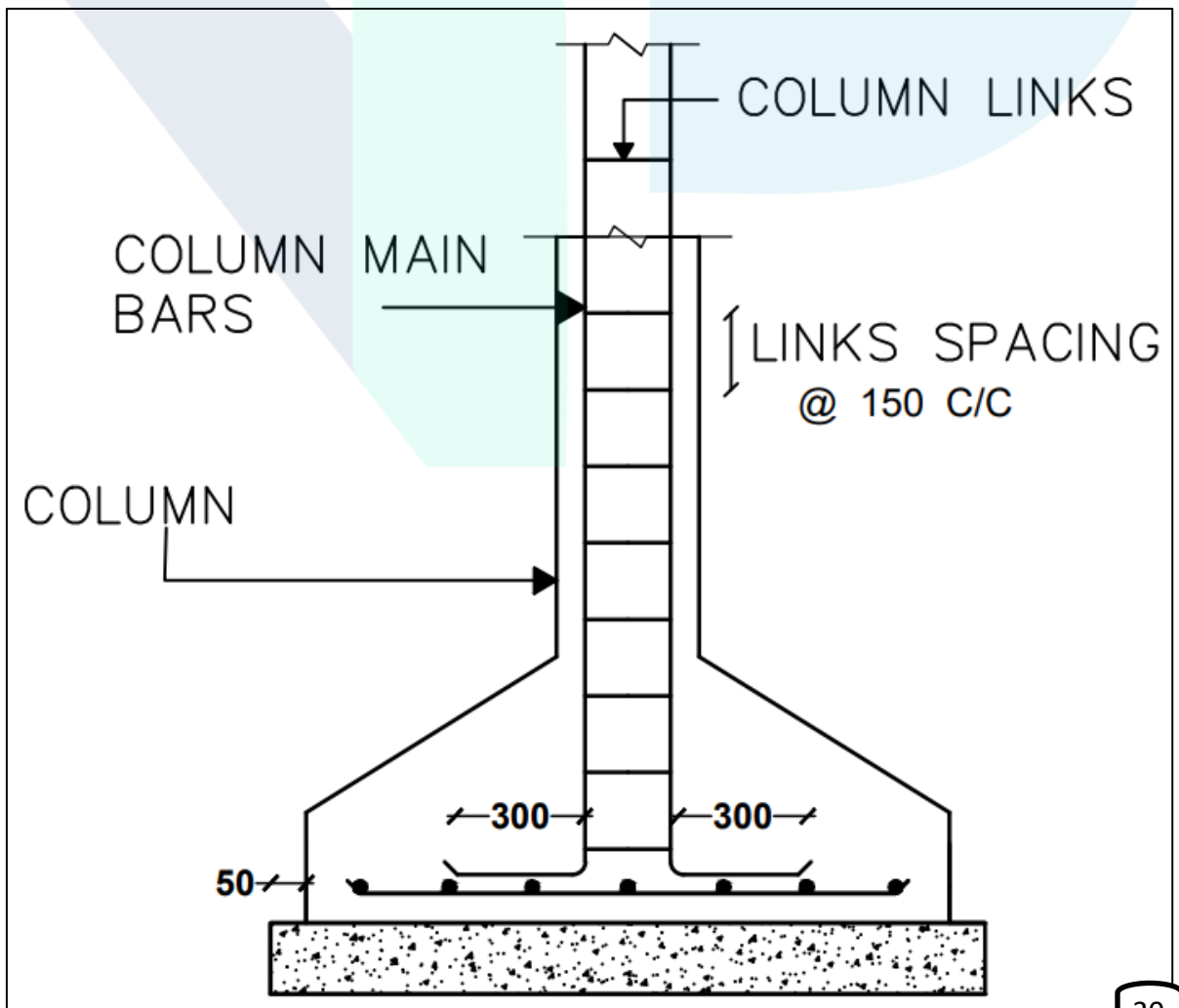
➤ Column & Reinforcement Details

COLUMN SIZE (m)	VERTICAL BARS	RINGS C/C	COLUMN SECTION AT G. FLOOR
0.2x0.45	4 X 12 4 X 10	8 ϕ @ 150 C/C	

➤ Column Layout Plan



➤ Column Section



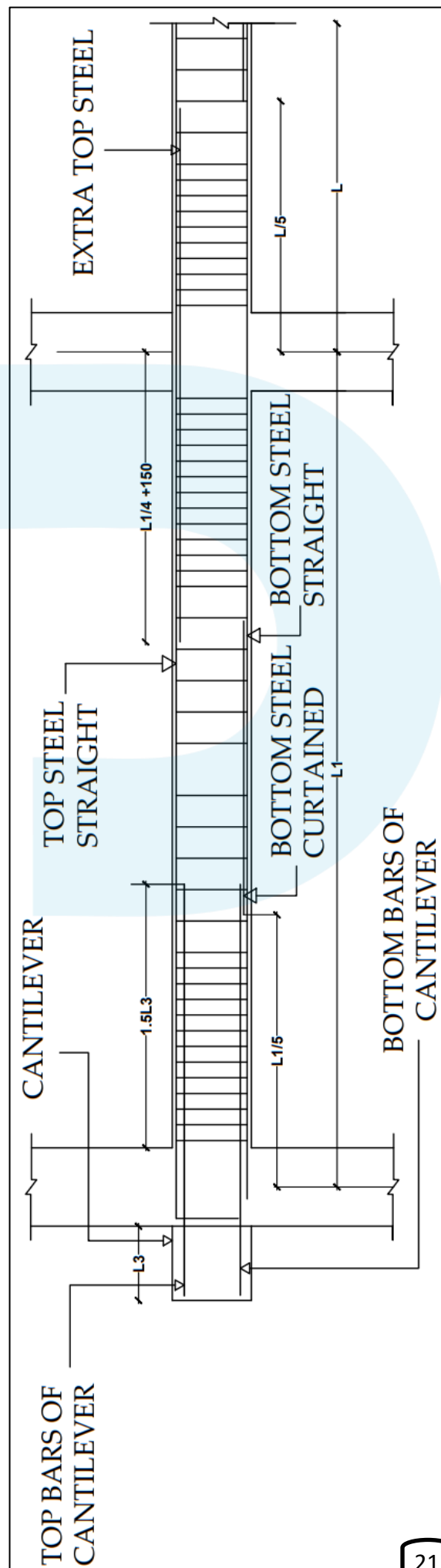
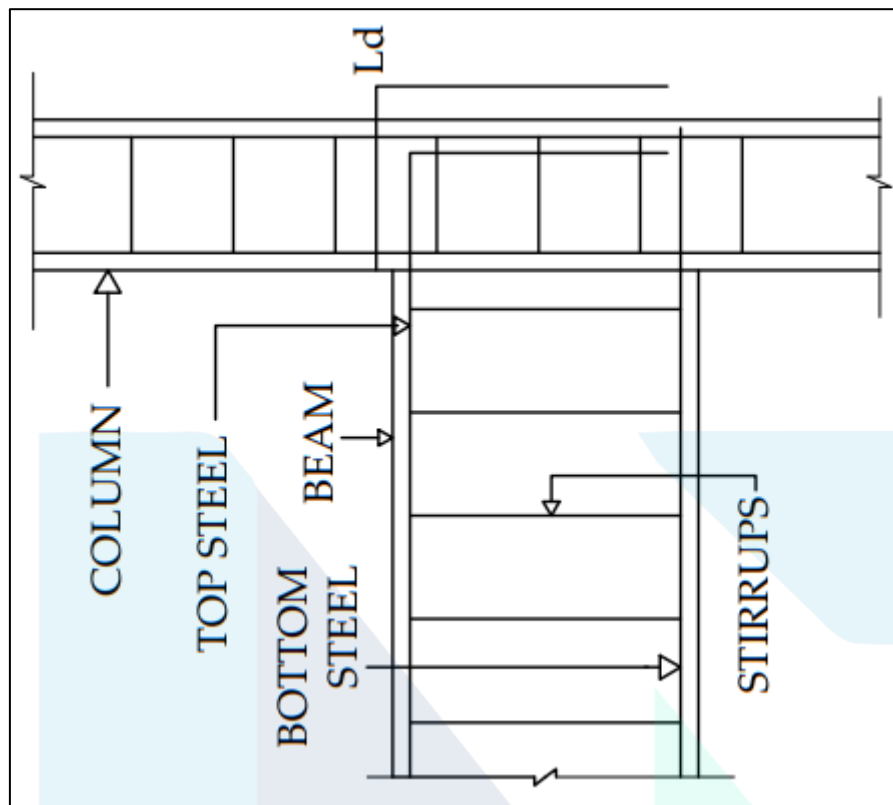
❖ Beam Drawing:

- Typical End Span Details of Beam

➤ Beam & Reinforcement Details

BEAM NO.	SIZE/THICKNESS BXD (OVERALL) M	MID SPAN MAIN BAR (MM)		TOP BARS (MM)	RINGS / DISTRIBUTION C/C (MM)
		BOTTOM STRAIGHT BARS	BOTTOM B/UP AT 2D FROM FACE		
B1	2 X 3	2 X 12 $\bar{\phi}$	1 X 10 $\bar{\phi}$	2 X 10	8 @ 150 C/C

➤ Details of Typical Continuous Beam



Development Length & Lap Length

Development Length/Anchorage Length (L_d):

- ❖ Development length refers to the length of reinforcement required to transfer the stresses between the concrete and the steel reinforcement bars to ensure adequate bond strength.
- ❖ It is essential because concrete has low tensile strength compared to steel, so the reinforcement bars need to be adequately anchored into the concrete to effectively resist tension and prevent slippage or pullout.
- ❖ The development length is critical in ensuring the stability and load-carrying capacity of reinforced concrete elements like beams, columns, and slabs.

Why Development Length is Used:

- ❖ When a load is applied to a reinforced concrete member, tensile stresses develop in the reinforcement bars. To prevent premature failure or debonding, the development length ensures that these stresses are safely transferred into the concrete.
- ❖ Proper development length is necessary to prevent bond failure, which could lead to structural instability and safety concerns.

Industry Value for Development Length:

- ❖ The value of development length depends on various factors, such as the type of bar, concrete strength, cover depth, and environmental conditions.
- ❖ Development length is specified in relevant design codes, such as ACI 318 (USA), BS 8110 (UK), IS 456 (India), and AS 3600 (Australia).

It is usually expressed in terms of the bar diameter (d) or as a multiple of the bar diameter (typically 40 times the bar diameter for straight bars in tension and 50 times the bar diameter for bent or hooked bars).

- ❖ Here are some useful details related to development length taken from Indian standard 456:200.

26.2.1 Development Length of Bars

The development length L_d is given by

$$L_d = \frac{\phi \sigma_s}{4\tau_{bd}}$$

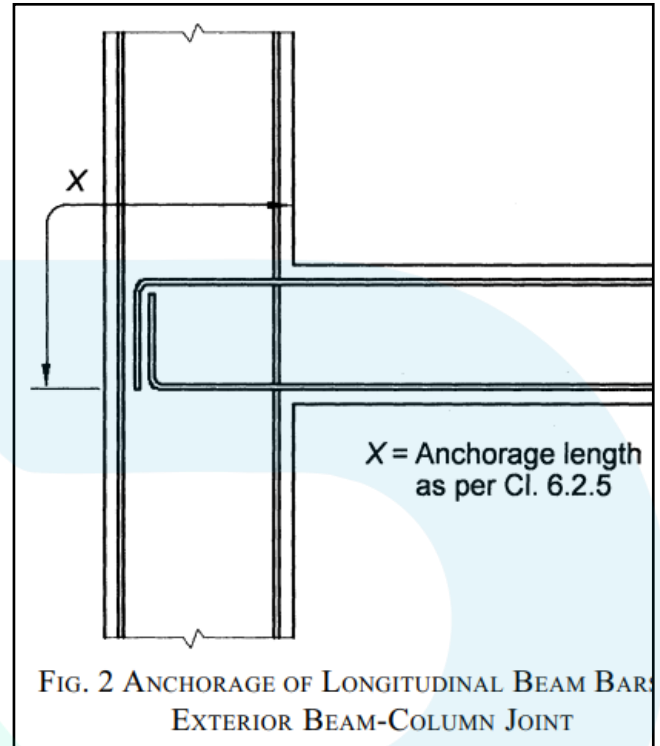
where

ϕ = nominal diameter of the bar,

σ_s = stress in bar at the section considered at design load, and

τ_{bd} = design bond stress given in 26.2.1.1.

Img.1.



Img.2.

26.2.1.1 Design bond stress in limit state method for plain bars in tension shall be as below:

Grade of concrete	M 20	M 25	M 30	M 35	M 40 and above
Design bond stress, τ_{bd} , N/mm ²	1.2	1.4	1.5	1.7	1.9

For deformed bars conforming to IS 1786 these values shall be increased by 60 percent.

For bars in compression, the values of bond stress for bars in tension shall be increased by 25 percent.

Img.3.

DEFORM BARS IN TENSION

Grade of concrete	M 20	M 25	M 30	M 35	M 40 and above
Design bond stress, τ_{bd} , N/mm ²	1.92	2.24	2.40	2.72	3.04

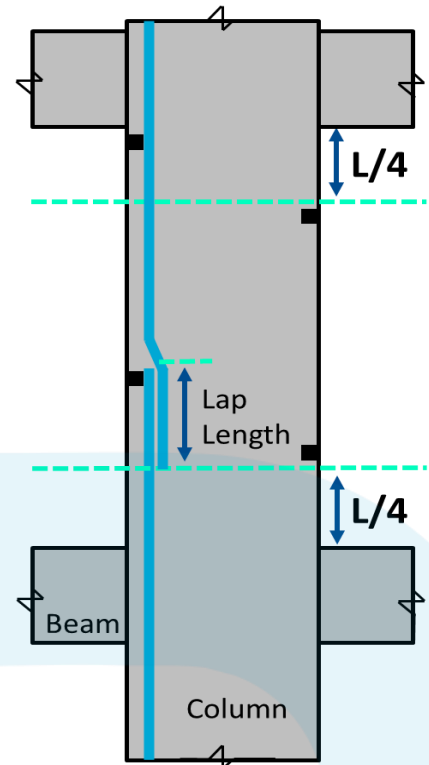
Img.4.

Lap Length:

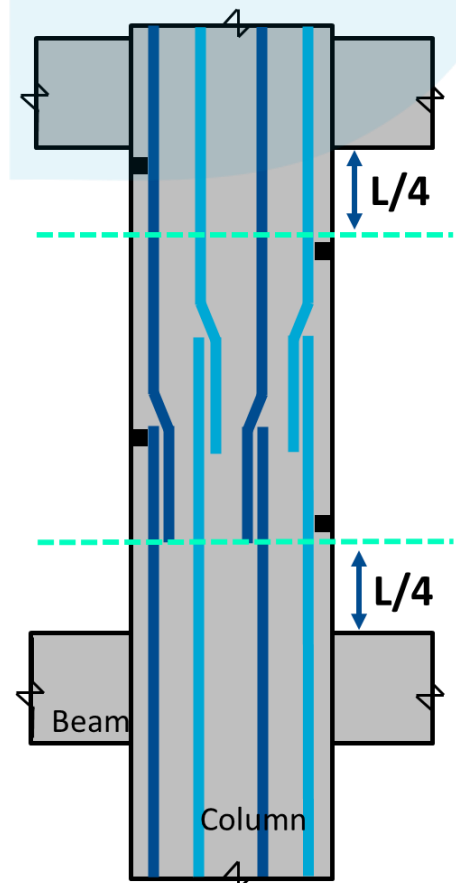
- ❖ Lap length is the amount of overlap or splice required for two reinforcement bars to develop adequate bond strength when they are joined together.
- ❖ It is essential to ensure the continuity of reinforcement along the length of the structure and to resist the tensile forces effectively.
- ❖ Lap length is used in locations where two reinforcement bars of the same diameter are joined together, like in beams, columns, and slabs.

Why Lap Length is Used:

- ❖ Lap length ensures the continuity of reinforcement, preventing any structural weakness or discontinuity in the concrete member.
- ❖ The lap length is crucial for distributing stresses uniformly across the splice, allowing the reinforcement to work as a single continuous element.



Lap Joint



Lap Joint with staggered pattern

Industry Value for Lap Length:

- ❖ Similar to development length, the lap length value depends on factors such as the bar diameter, concrete strength, cover depth, and design codes.
- ❖ The lap length is specified in design codes, such as ACI 318 (USA), BS 8110 (UK), IS 456 (India), and AS 3600 (Australia).
- ❖ It is typically expressed as a multiple of the bar diameter or as an absolute length. Lap length should not be less than the development length.
- ❑ Please note that the specific values for development length and lap length can vary depending on the structural design requirements and the code used in a particular country. Engineers and designers should refer to the relevant design codes for accurate and appropriate values of development length and lap length in their projects.

6.2.6 Splicing of Longitudinal Bars

6.2.6.1 Lap splices

When adopted, closed links shall be provided over the entire length over which the longitudinal bars are spliced. Further,

- a) the spacing of these links shall not exceed 150 mm (see Fig. 3).
- b) the lap length shall not be less than the development length of the largest longitudinal reinforcement bar in tension.
- c) lap splices shall not be provided,
 - 1) within a joint ;
 - 2) within a distance of $2d$ from face of the column; and

**Data taken from:
IS 13920:2016**



Cutting Length Formula in BBS

Cutting Length Formula



Shape



Cutting Length

$$CL = L$$



Shape



Cutting Length

$$CL = a + L + a - \text{Bend Deduction}$$

$$\text{Bend Deduction : } 2 \times 2D (90^\circ)$$



Shape



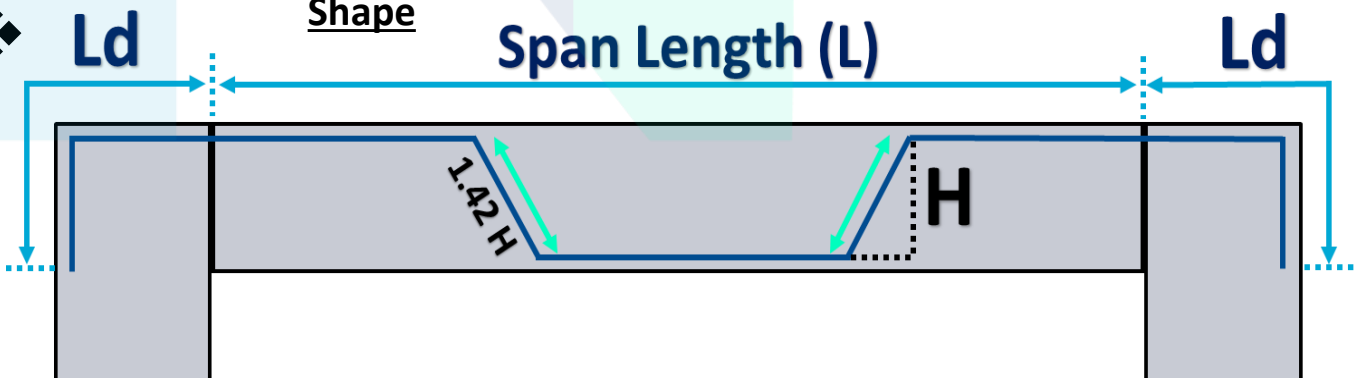
Cutting Length

$$CL = a + L - \text{Bend Deduction}$$

$$\text{Bend Deduction : } 1 \times 2D (90^\circ)$$



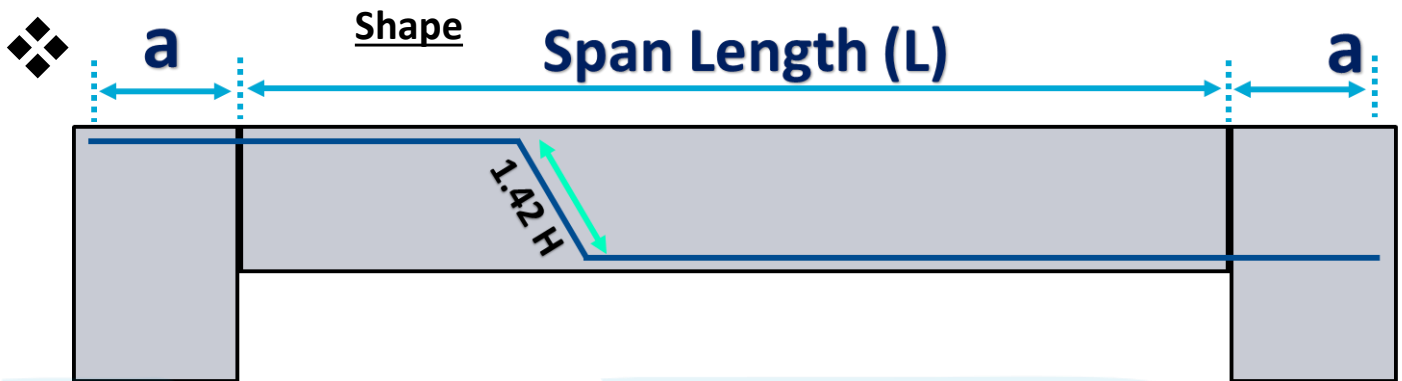
Shape



$$CL = Ld + \text{Span Length}(L) + (0.42H \times 2) + Ld - \text{Bend Deduction (2x90 + 4x45)}$$

- ❑ Note: This formula is for curtail/crank bar in beam. As you can see in diagram the actual value of crank part is '1.42H', but in the time of calculation we derive the standard value to calculate CL without complication which is '0.42H'. Derivation is in next part.

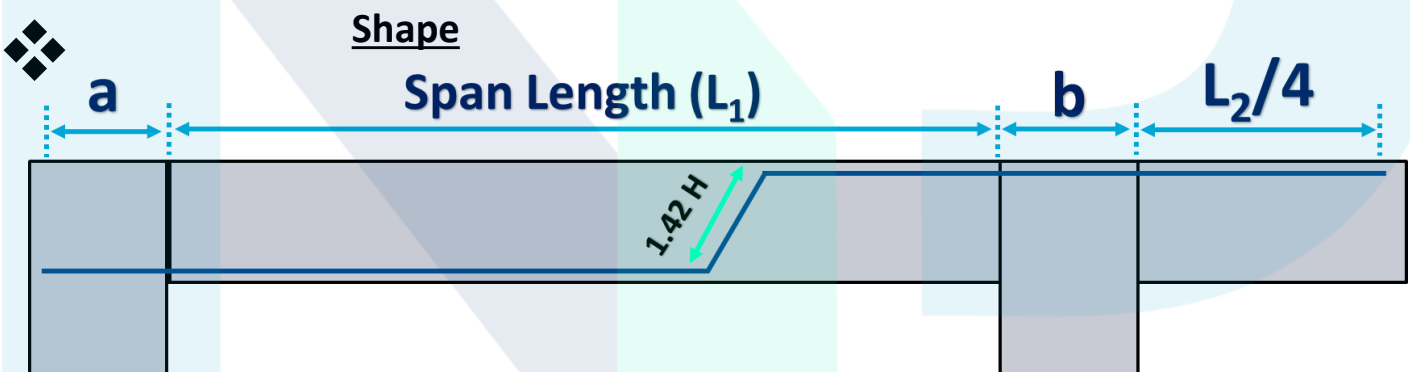
Cutting Length Formula



$$CL = a + \text{Span Length}(L) + (0.42H) + a - \text{Bend Deduction (2x45)}$$

(Where, a = Beam width – Cover)

- ❑ Note: This formula is for curtail/crank bar in Slab. As you can see in diagram the actual value of crank part is '1.42H', but in the time of calculation we derive the standard value to calculate CL without complication which is '0.42H'. Derivation is in next part.



$$CL = a + \text{Span Length}(L_1) + (0.42H) + b + L_2/4 - \text{Bend Deduction (2x45)}$$

(Where, a = Beam width – Cover)

- ❑ Note: This formula is for curtail/crank bar in Continuous Slab. As you can see in diagram the actual value of crank part is '1.42H', but in the time of calculation we derive the standard value to calculate CL without complication which is '0.42H'. Derivation is in next part.

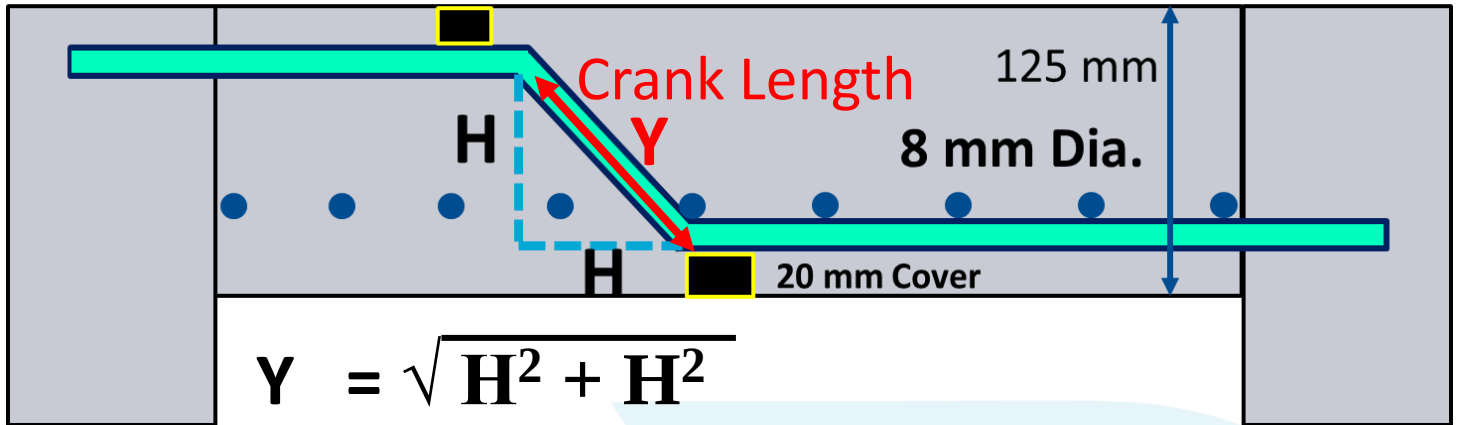
❖ Formula To Calculate Number of Bars:

1) When bar is calculated from start to end point = $\frac{\text{Distance}}{\text{Spacing}} + 1$

2) When bar is calculated from start but not till end point = $\frac{\text{Distance}}{\text{Spacing}}$

3) When bar is calculated excluding start and end point = $\frac{\text{Distance}}{\text{Spacing}} - 1$

Crank Length Calculation



$$Y = \sqrt{H^2 + H^2}$$

$$Y = \sqrt{2H^2}$$

$$Y = \sqrt{2} H$$

$$Y = 1.42 H$$

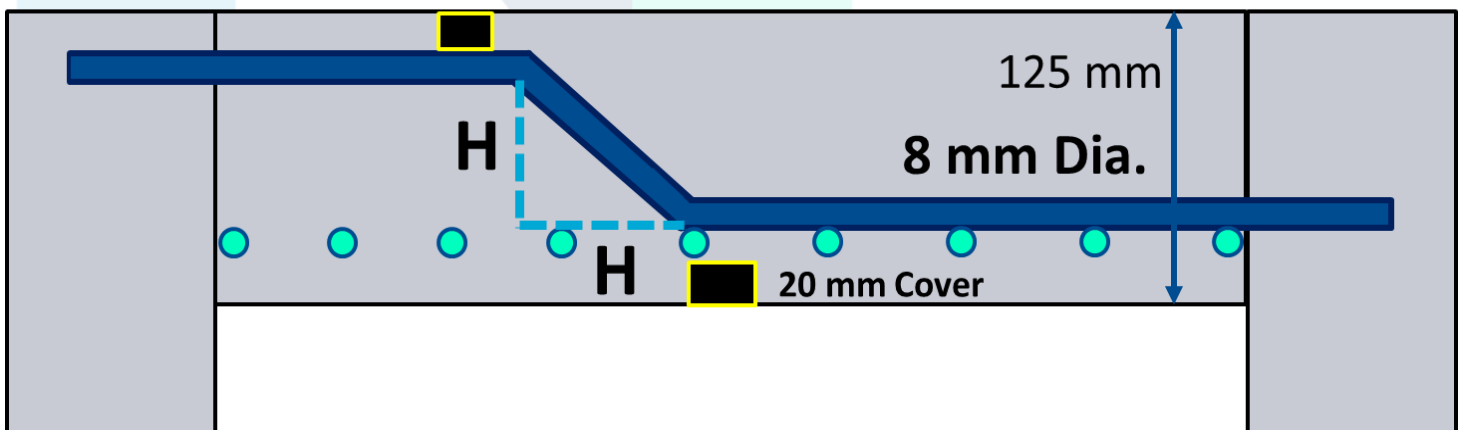
- Note: This formula is for curtail/crank bar in slab/beam. As you can see in diagram the actual value of crank part is '1.42H', but in the time of calculation we calculate it as standard value of '0.42H', because it is easier to calculate this way.



Formula to calculate value of 'H' when bar is placed at bottom

$H = \text{Slab Depth} - (2 \times \text{Cover}) - \text{Dia. of bar}$

$$H = 125 - (2 \times 20) - 8 = 77 \text{ mm}$$



Formula to calculate value of 'H' when bar is placed just above bottom bar

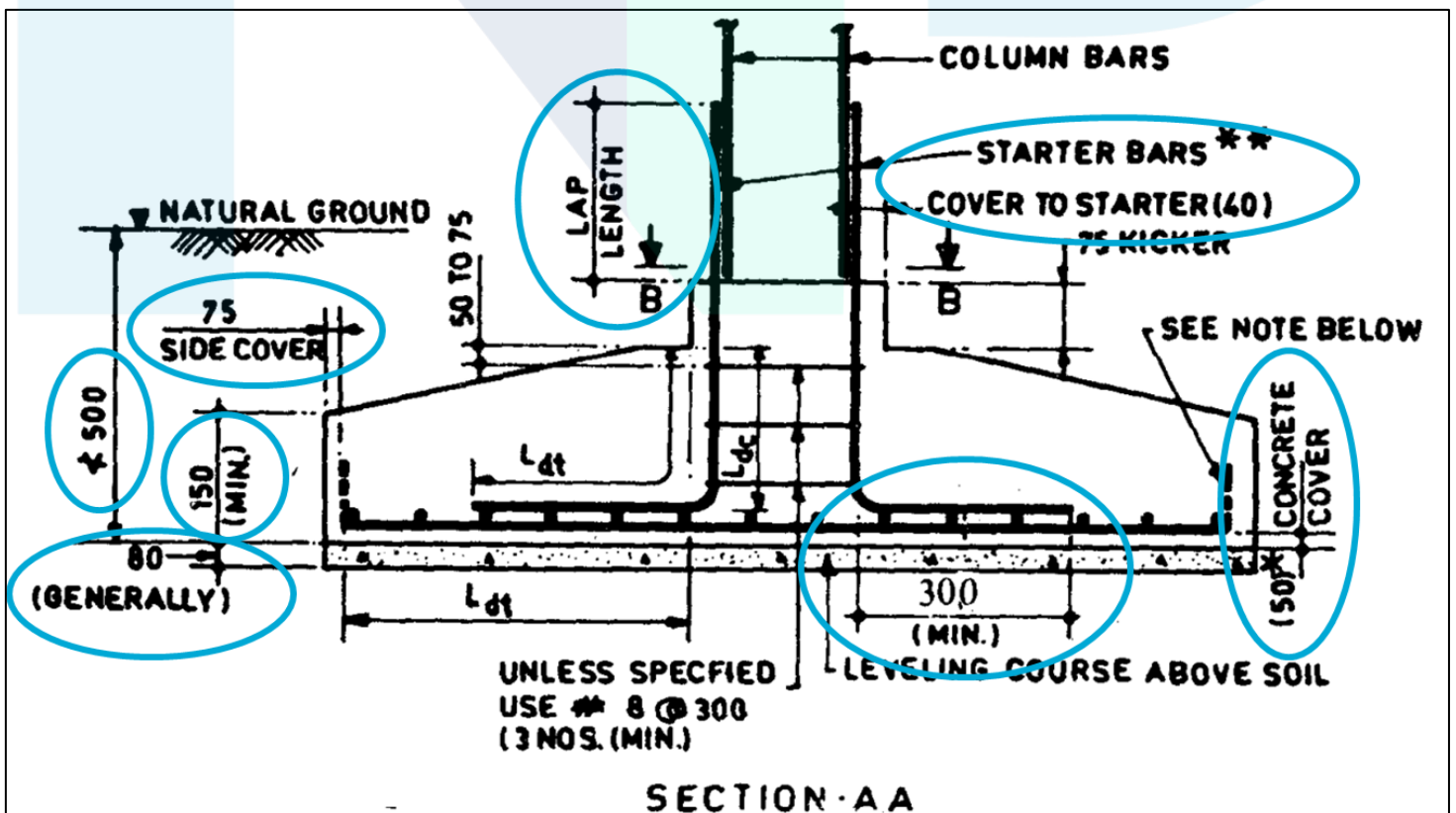
$H = \text{Slab Depth} - (2 \times \text{Cover}) - \text{Dia. of bar} - \text{Dia. Of bottom bar}$

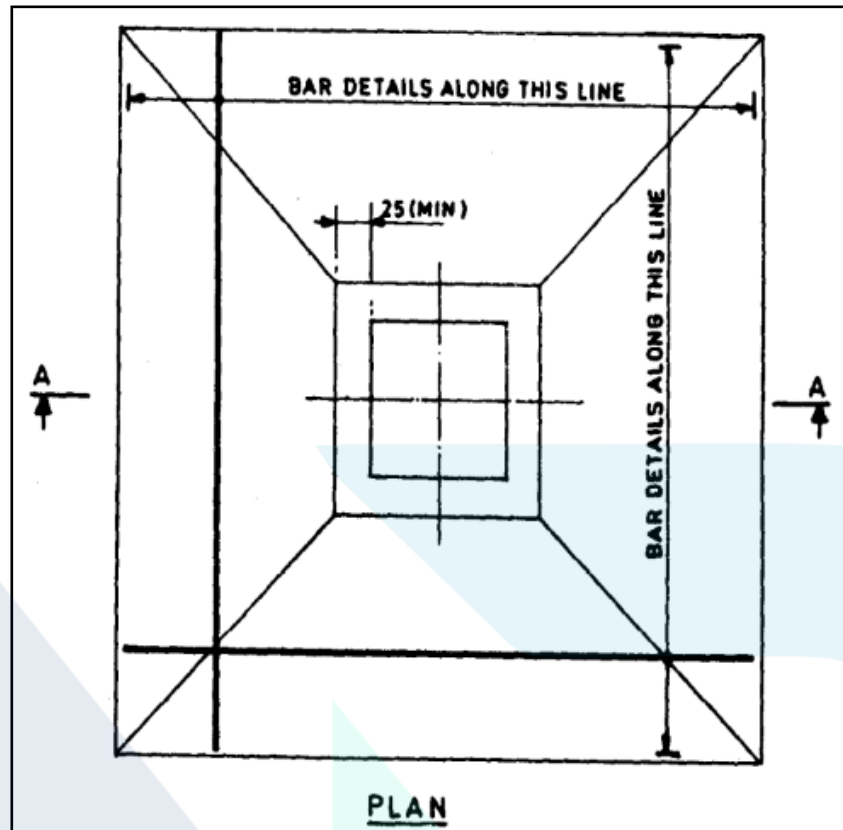
$$H = 125 - (2 \times 20) - 8 - 8 = 69 \text{ mm}$$

IS Codes Used in BBS



Concrete Reinforcement & Detailing SP-34:1987





6.2 Cover — The minimum thickness of cover to main reinforcement shall not be less than 50 mm for surfaces in contact with earth face and not less than 40 mm for external exposed face. However, where the concrete is in direct contact with the soil, for example, when a levelling course of lean concrete is not used at the bottom of footing, it is usual to specify a cover of 75 mm. This allows for the uneven surface of the excavation. In case of raft foundation, whether resting directly on soil or on lean concrete, the cover for the reinforcement shall not be less than 75 mm.

6.3 Minimum Reinforcement and Bar Diameter — The minimum reinforcement according to slab and beam elements as appropriate should be followed, unless otherwise specified. The diameter of main reinforcing bars should be not less than 10 mm.



Plain & Reinforced Concrete Code of Practice IS 456:2000

26.4.1 Nominal Cover

26.4.2.1 However for a longitudinal reinforcing bar in a column nominal cover shall in any case not be less than 40 mm, or less than the diameter of such bar. In the case of columns of minimum dimension of 200 mm or under, whose reinforcing bars do not exceed 12 mm, a nominal cover of 25 mm may be used.

26.5.3 Columns

26.5.3.1 Longitudinal reinforcement

- c) The minimum number of longitudinal bars provided in a column shall be four in rectangular columns and six in circular columns.
- d) The bars shall not be less than 12 mm in diameter.
- e) A reinforced concrete column having helical reinforcement shall have at least six bars of longitudinal reinforcement within the helical reinforcement.

26.2.5.1 Lap splices

- a) Lap splices shall not be used for bars larger than 36 mm; for larger diameters, bars may be welded (see 12.4); in cases where welding is not practicable, lapping of bars larger than 36 mm may be permitted, in which case additional spirals should be provided around the lapped bars.
- b) Lap splices shall be considered as staggered if the centre to centre distance of the splices is not less than 1.3 times the lap length calculated as described in (c).

- d) The lap length in compression shall be equal to the development length in compression, calculated as described in 26.2.1, but not less than 24 ϕ .
- e) When bars of two different diameters are to be spliced, the lap length shall be calculated on the basis of diameter of the smaller bar.

Ductile Detailing Of Reinforced Concrete Structures Subjected To Seismic Forces -Code Of Practice IS 13920:1993

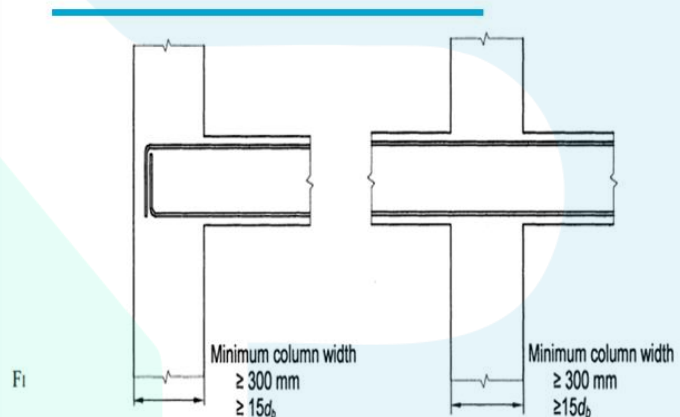
7 COLUMNS AND FRAME MEMBERS SUBJECTED TO BENDING AND AXIAL LOAD

7.1.2 The minimum dimension of the member shall not be less than 200 mm. However, in frames which have beams with centre to centre span exceeding 5 m or columns of unsupported length exceeding 4 m, the shortest dimension of the column shall not be less than 300 mm.

7.1.3 The ratio of the shortest cross sectional dimension to the perpendicular dimension shall preferably not be less than 0.4.

7.1.1 The minimum dimension of a column shall not be less than,

- $20 d_b$, where d_b is diameter of the largest diameter longitudinal reinforcement bar in the beam passing through or anchoring into the column at the joint, or
- 300 mm (see Fig. 7).



7.3 Longitudinal Reinforcement

7.3.1 Circular columns shall have minimum of 6 bars.

7.3.2 Splicing of Longitudinal Bars

7.3.2.1 Lap splices

When adopted, closed links shall be provided over the entire length over which the longitudinal bars are spliced. Further,

- the spacing of these links shall not exceed 100 mm.
- the lap length shall not be less than the development length of the largest longitudinal reinforcement bar in tension.
- lap splices shall be provided only in the central half of clear column height, and not
 - within a joint, or
 - within a distance of $2d$ from face of the beam.
- not more than 50 percent of area of steel bars shall be spliced at any one section.
- lap splices shall not be used for bars of diameter larger than 32 mm for which mechanical splicing shall be adopted.

7.4 Transverse Reinforcement

7.4.1 Transverse reinforcement shall consist of closed loop,

- spiral or circular links in circular columns, and
- rectangular links in rectangular columns.

In either case, the closed link shall have 135° hook ends with an extension of 6 times its diameter (but not < 65 mm) at each end, which are embedded in the confined core of the column (see Fig. 10A).

7.4.2 When rectangular links are used,

- the minimum diameter permitted of transverse reinforcement bars is 8 mm, when diameter of longitudinal bar is less than or equal to 32 mm, and 10 mm, when diameter of longitudinal bar is more than 32 mm;
- the maximum spacing of parallel legs of links shall be 300 mm centre to centre;



Ductile Detailing Of Reinforced Concrete Structures Subjected To Seismic Forces -Code Of Practice IS 13920:1993

6 BEAMS

6.1 General

Requirements of this section shall apply to beams resisting earthquake-induced effects, in which the factored axial compressive stress does not exceed $0.08 f_{ck}$. Beams, in which the factored axial compressive stress exceeds $0.08 f_{ck}$, shall be designed as per requirements of 7.

6.1.1 Beams shall preferably have width-to-depth ratio of more than 0.3.

6.1.2 Beams shall not have width less than 200 mm.

6.1.3 Beams shall not have depth D more than 1/4th of clear span. This may not apply to the floor beam of frame staging of elevated RC water tanks.

6.2 Longitudinal Reinforcement

6.2.1 The longitudinal reinforcement in beams shall be as given below:

- Beams shall have at least two 12 mm diameter bars each at the top and bottom faces.

6.2.5 At an exterior joint, top and bottom bars of beams shall be provided with anchorage length beyond the inner face of the column, equal to development length of the bar in tension plus 10 times bar diameter minus the allowance for 90° bends (see Fig. 2).

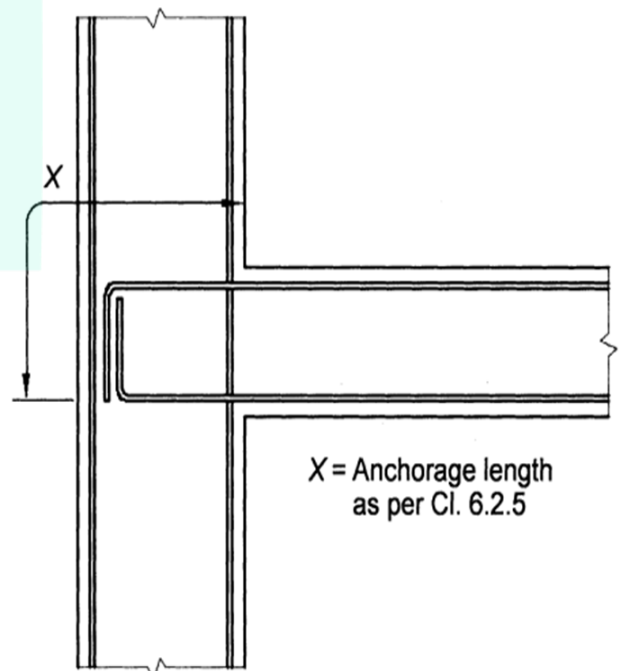


FIG. 2 ANCHORAGE OF LONGITUDINAL BEAM BARS AT EXTERIOR BEAM-COLUMN JOINT

6.2.6 Splicing of Longitudinal Bars

6.2.6.1 Lap splices

When adopted, closed links shall be provided over the entire length over which the longitudinal bars are spliced. Further,

- the spacing of these links shall not exceed 150 mm (see Fig. 3).
- the lap length shall not be less than the development length of the largest longitudinal reinforcement bar in tension.
- lap splices shall not be provided,
 - within a joint ;
 - within a distance of $2d$ from face of the column; and
 - within a quarter length of the beam adjoining the location where flexural yielding may occur under earthquake effects.

6.3.2 The minimum diameter of a link shall be 8 mm.

- not more than 50 percent of area of steel bars on either top or bottom face shall be spliced at any one section.

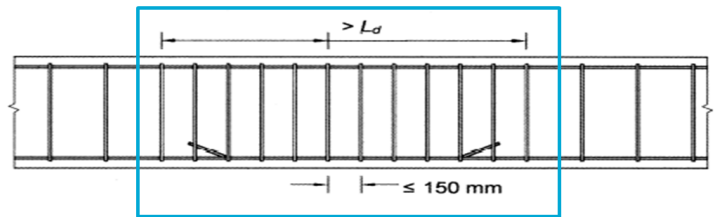


FIG. 3 LAP LENGTH AT LOCATION OF SPLICING OF LONGITUDINAL BARS IN BEAM

6.3.5 Close Spacing of Links

Spacing of links over a length of $2d$ at either end of a beam shall not exceed,

- $d/4$;
- 8 times the diameter of the smallest longitudinal bar; and
- 100 mm (see Fig. 6).

6.3.5.1 The first link shall be at a distance not exceeding 50 mm from the joint face.

6.3.5.2 Closely spaced links shall be provided over a length equal to $2d$ on either side of a section where flexural yielding may occur under earthquake effects. Over the remaining length of the beam, vertical links shall be provided at a spacing not exceeding $d/2$.

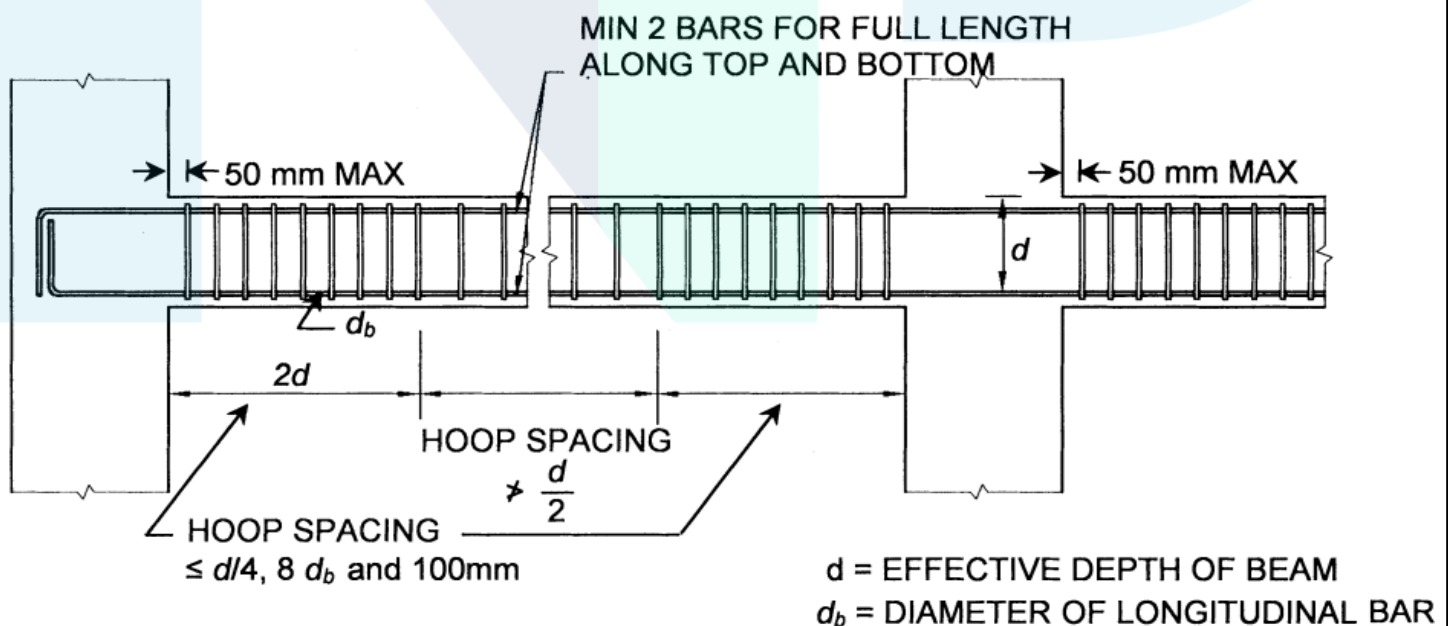


FIG. 6 DETAILS OF TRANSVERSE REINFORCEMENT IN BEAMS



Concrete Reinforcement & Detailing SP-34:1987

9.2 Spacing, Cover and Diameter

9.2.1 Spacing

- The pitch of the bars for main tensile reinforcement in solid slab shall be not more than thrice the effective depth of such slab or 450 mm, whichever is smaller.
- The pitch of the distribution bars or the pitch of the bars provided against shrinkage and temperature shall not be more than 5 times the effective depth of such slab or 450 mm, whichever is smaller. Table C-6 (see Appendix C) give area of bars for different spacing and diameter of bars.

9.2.2 Cover

- The cover at each end of reinforcing bar shall be neither less than 25 mm nor less than twice the diameter of such bar.
- The minimum cover to reinforcement (tension, compression, shear) shall be not less than 15 mm, nor less than the diameter of bar.

9.2.3 Bar Diameters—The main bars in the slab shall not be less than 8 mm (high yield strength bars) or 10 mm (plain bars) and distribution steel shall not be less than 6 mm diameter bars. The diameter of the bar shall not also be more than one-eighth of the slab thickness.

Plain & Reinforced Concrete Code of Practice IS 456:2000

D-1.4 Tension reinforcement provided at mid-span in the middle strip shall extend in the lower part of the slab to within $0.25 l$ of a continuous edge, or $0.15 l$ of a discontinuous edge.

D-1.5 Over the continuous edges of a middle strip, the tension reinforcement shall extend in the upper part of the slab a distance of $0.15 l$ from the support, and at least 50 percent shall extend a distance of $0.3 l$.

BBS of RCC Component

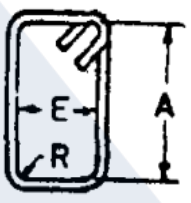
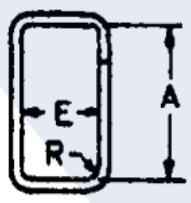
Stirrups

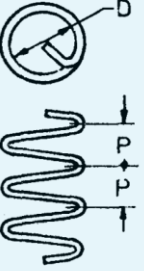
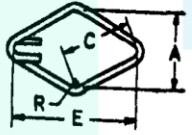
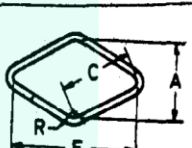
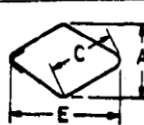
IS Code Reference of Stirrup IS: 2502 - 19631

IS : 2502 - 1963

TABLE VIII MEASUREMENT OF BENDING DIMENSIONS FOR BINDERS, STIRRUPS, LINKS AND THE LIKE FOR REINFORCED CONCRETE

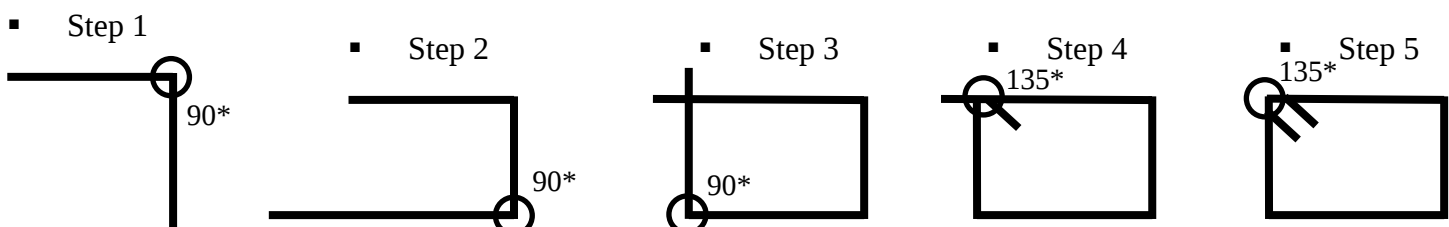
(Clauses 3.1 and 3.1.2)

REF No.	METHOD OF MEASUREMENT OF BENDING DIMENSION	APPROX TOTAL LENGTH OF BAR (L) MEASURED ALONG CENTRE LINE	SKETCH AND DIMENSIONS TO BE GIVEN IN SCHEDULE
A		$2(A + E) + 24d$	 (See Notes 1 and 3)
B		$2(A + E) + 20d$	 (See Notes 1 and 3)

	Where P is not greater than $D/5$ $N \pi (D + d) + 8d$ N = number of complete and fractional turns D = internal dia P = pitch of helix d = size of bar	F 	$4C + 24d$	 (See Notes 1 and 3)
		G 	$4C + 20d$	 (See Notes 1 and 3)

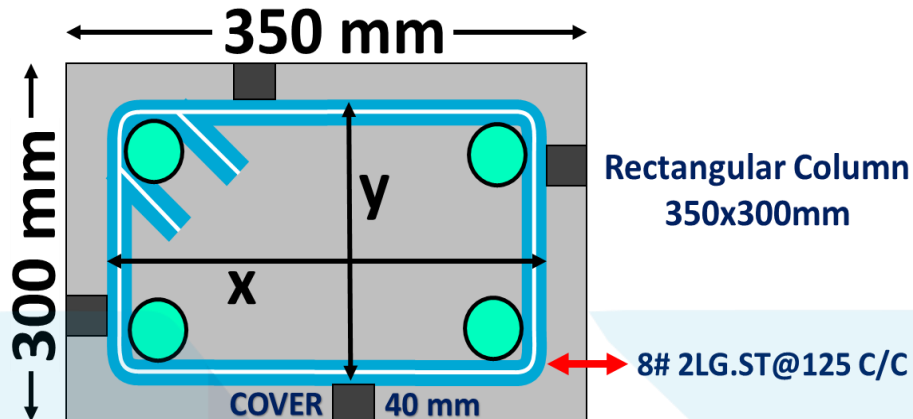
Note: Bend deduction while calculating cutting length depends upon the company pattern. So, follow as per company requirement. These calculation is for understanding purpose with bend deduction.

❖ Steps Involved in making a stirrup



Bar Bending Schedule of Stirrup

❖ Rectangular Stirrup

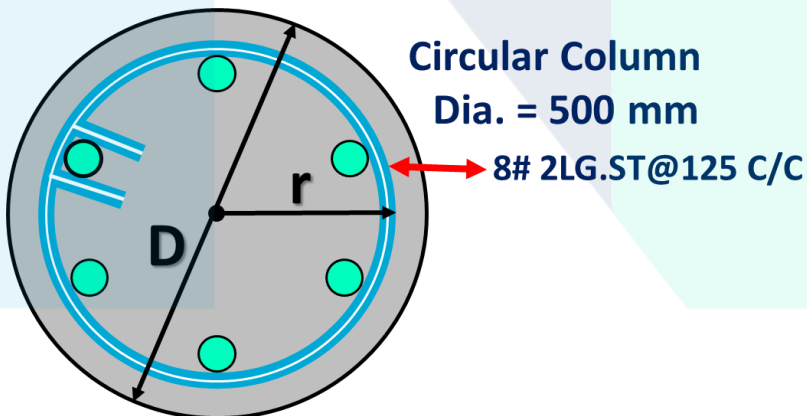


❑ Follow these value

- Hook Length
90° Hook = 10D
135° Hook = 12D
- Bend Deduction
90° Bend = 2D
135° Bend = 3D

- Cutting Length Formula = $2(x + y) + \text{Hook Length} - \text{Bend Deduction}$
- Cutting Length Formula = $2(x + y) + (2 \times 135^\circ \text{hook}) - (3 \times 90^\circ \text{bend}) - (2 \times 135^\circ \text{bend})$
- Where, $x = 350 - 40 - 40 = 270 \text{ mm}$, $y = 300 - 40 - 40 = 220 \text{ mm}$
- Cutting Length = $2(270 + 220) + 2(12 \times 8) - (3 \times 2 \times 8) - (2 \times 3 \times 8)$
= Answer solve yourself

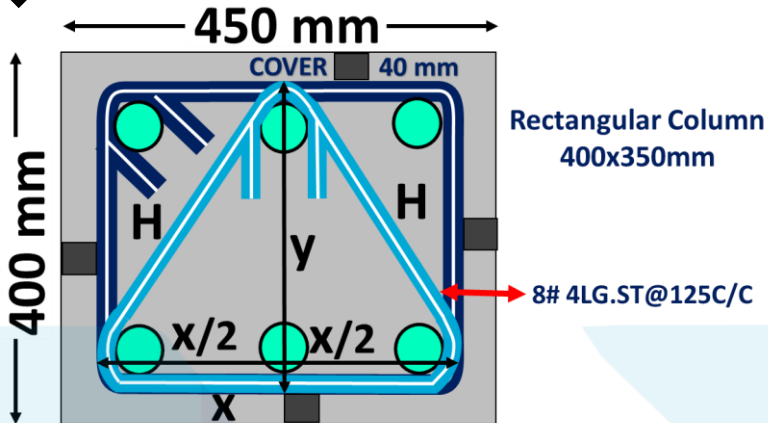
❖ Circular Stirrup



- Cutting Length Formula : $2\pi r + \text{Hook Length} - \text{Bend Deduction}$
- Cutting Length Formula : $2\pi r + (2 \times 90^\circ \text{hook}) - (2 \times 90^\circ \text{bend})$
- $r = D/2 - \text{Cover}$
- $r = 500/2 - 40 = 210 \text{ mm}$
- Cutting Length: $2 \times \pi \times 210 + (2 \times 10 \times 8) - (2 \times 2 \times 8)$
= Answer solve yourself

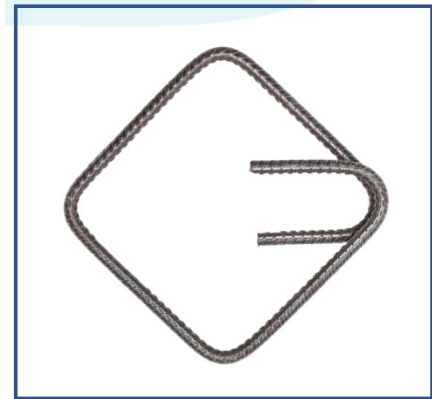
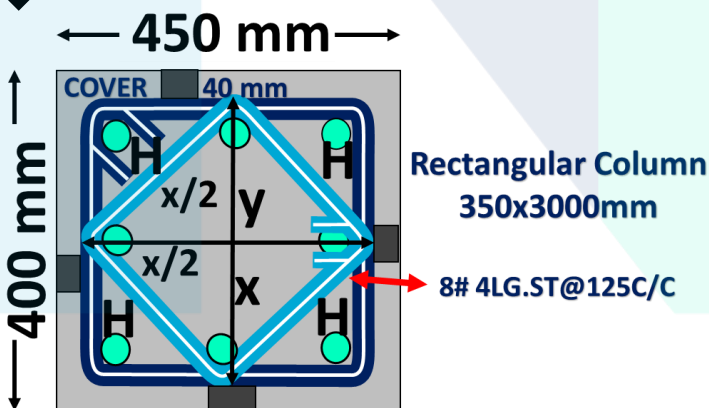
Bar Bending Schedule of Stirrup

❖ Triangular Stirrup



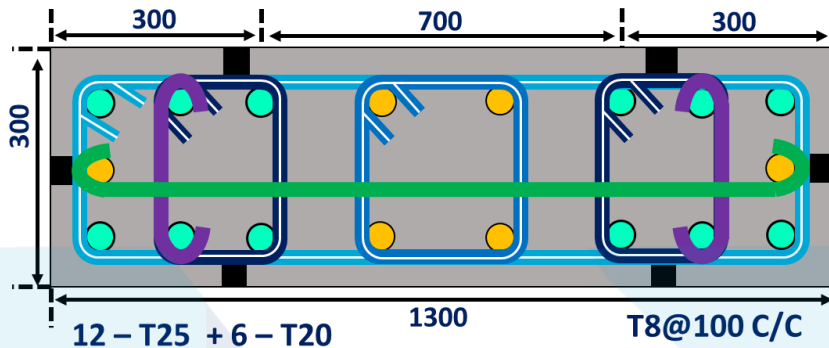
- Cutting Length Formula : $(2H + X) + \text{Hook Length} - \text{Bend Deduction}$
- Cutting Length Formula : $(2H + X) + (2 \times 135 \times \text{hook}) - (4 \times 135 \times \text{bend})$
- $H = \sqrt{(X/2)^2 + (Y)^2}$, $X = 450 - 40 - 40 = 370 \text{ mm}$, $Y = 400 - 40 - 40 = 320 \text{ mm}$
- $H = \sqrt{(370/2)^2 + (320)^2}$ $H = 370 \text{ mm}$
- Cutting Length Formula : $(2 \times 370 + 370) + (2 \times 12 \times 8) - (4 \times 3 \times 8)$
= Answer solve yourself

❖ Diamond Stirrup

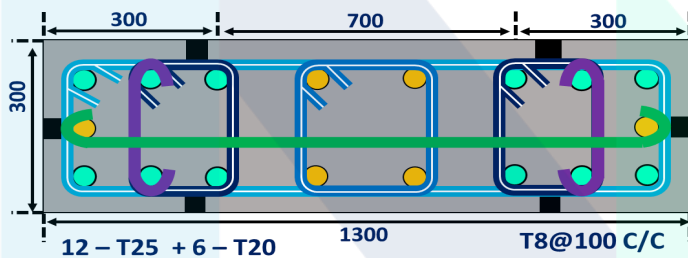


- Cutting Length Formula : $(4H) + \text{Hook Length} - \text{Bend Deduction}$
- Cutting Length Formula : $(4H) + (2 \times 135 \times \text{hook}) - (3 \times 90 \times \text{bend}) - (2 \times 135 \times \text{bend})$
- $H = \sqrt{(X/2)^2 + (Y/2)^2}$, $X = 450 - 40 - 40 = 370 \text{ mm}$, $Y = 400 - 40 - 40 = 320 \text{ mm}$
- $H = \sqrt{(370/2)^2 + (320/2)^2}$ $H = 245 \text{ mm}$
- Cutting Length Formula : $(4 \times 245) + (2 \times 12 \times 8) - (3 \times 2 \times 8) - (2 \times 3 \times 8)$
= Answer solve yourself

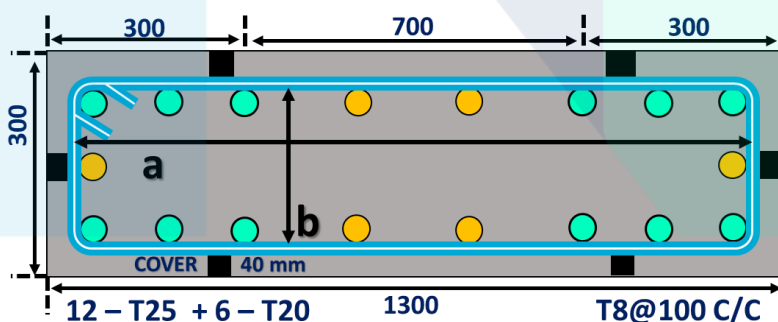
❖ Multiple Binders



❖ How Multiple Binders are placed in column



❖ Calculating Cutting Length: STEP - 1



Where,

$$a = 1300 - 40 - 40 = 1220 \text{ mm}$$

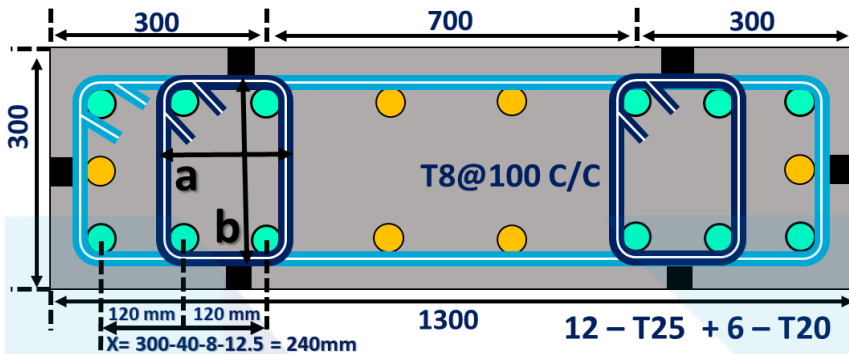
$$b = 300 - 40 - 40 = 220 \text{ mm}$$

- Cutting Length (CL) : $2(a+b) + \text{Hook Length} - \text{Bend Deduction}$
- Cutting Length (CL) : $2(a+b) + (2 \times 135 \times \text{hook}) - (3 \times 90 \times \text{bend}) - (2 \times 135 \times \text{bend})$
- Cutting Length (CL) : $2 (1220 + 220) + (2 \times 12 \times 8) - (3 \times 2 \times 8) - (2 \times 3 \times 8)$

= Answer solve yourself

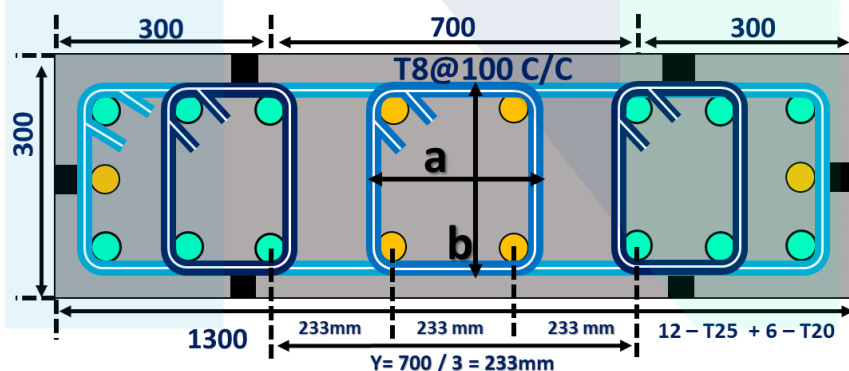
❖ Multiple Binders

❖ Calculating Cutting Length: STEP - 2



- Cutting Length (CL) : $2(a+b) + \text{Hook Length} - \text{Bend Deduction}$
 - Cutting Length (CL) : $2(a+b) + (2 \times 135 \times \text{hook}) - (3 \times 90 \times \text{bend}) - (2 \times 135 \times \text{bend})$
 - Cutting Length (CL) : $2 (161 + 220) + (2 \times 12 \times 8) - (3 \times 2 \times 8) - (2 \times 3 \times 8)$
- = Answer solve yourself

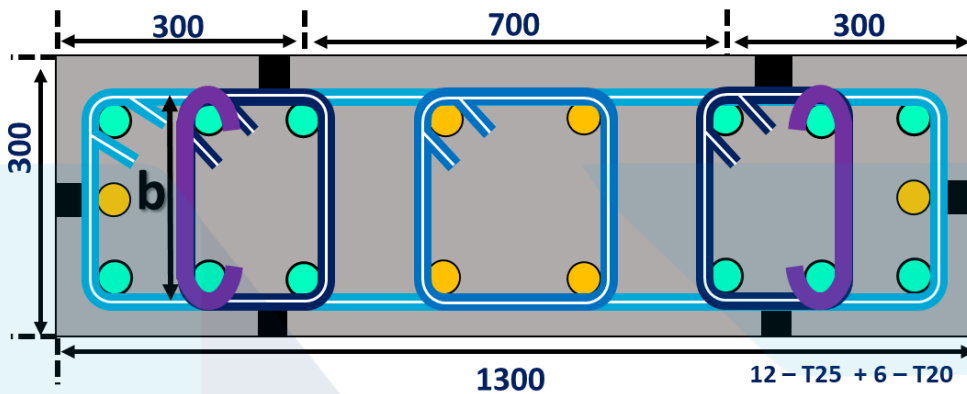
❖ Calculating Cutting Length: STEP - 3



- Cutting Length (CL) : $2(a+b) + \text{Hook Length} - \text{Bend Deduction}$
 - Cutting Length (CL) : $2(a+b) + (2 \times 135 \times \text{hook}) - (3 \times 90 \times \text{bend}) - (2 \times 135 \times \text{bend})$
 - Cutting Length (CL) : $2 (269 + 220) + (2 \times 12 \times 8) - (3 \times 2 \times 8) - (2 \times 3 \times 8)$
- = Answer solve yourself

❖ Multiple Binders

❖ Calculating Cutting Length: STEP - 4

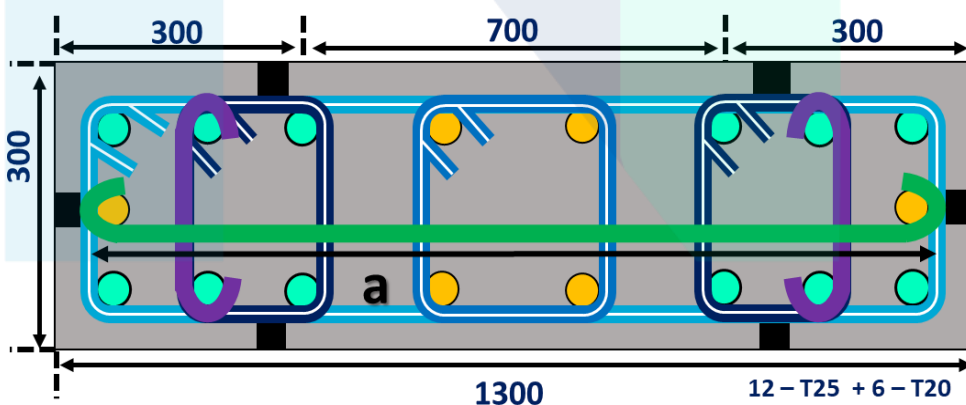


Where,

$$b = 300 - 40 - 40 = 220 \text{ mm}$$

- Cutting Length (CL) : $b + \text{Hook Length}$
 - Cutting Length (CL) : $b + (2 \times 180 \times \text{hook})$
 - Cutting Length (CL) : $220 + (2 \times 12 \times 8)$
- = Answer solve yourself

❖ Calculating Cutting Length: STEP - 5



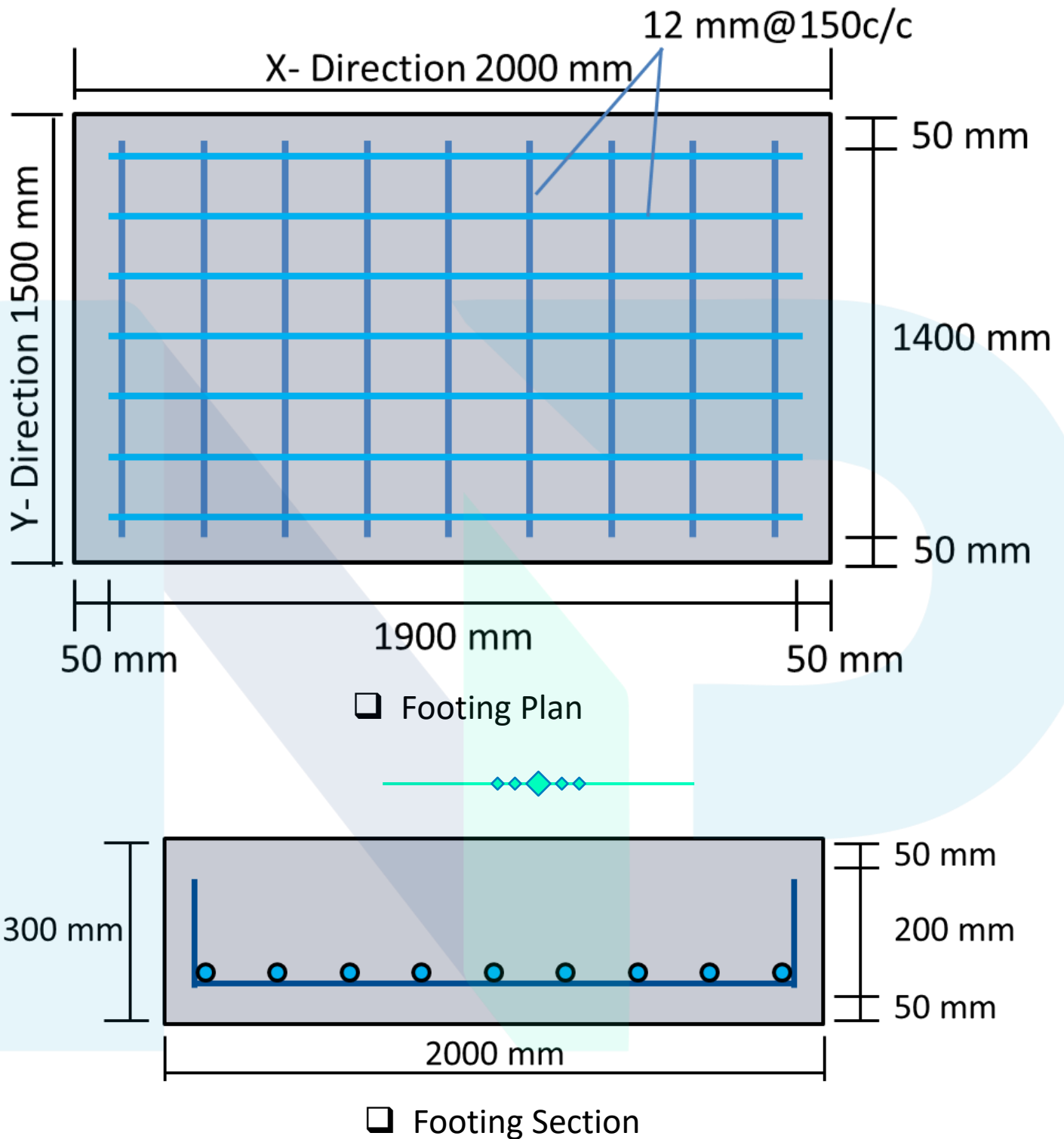
Where,

$$a = 1300 - 40 - 40 = 1220 \text{ mm}$$

- Cutting Length (CL) : $a + \text{Hook Length}$
 - Cutting Length (CL) : $a + (2 \times 180 \times \text{hook})$
 - Cutting Length (CL) : $1220 + (2 \times 12 \times 8)$
- = Answer solve yourself

Footing

Bar Bending Schedule of Footing



Given data:

- Footing length = 2000 mm, width = 1500 mm, depth = 300 mm
- Rebar diameter = 12 mm
- Spacing=150 mm
- Cover = 50 mm on all the sides

Bar Bending Schedule of Footing

□ No. of bars along x-direction

- = $[\{ (\text{footing length}) - (2 \times \text{cover}) \} \div \text{spacing}] + 1$
- = $[\{ (2000 \text{ mm}) - (2 \times 50 \text{ mm.}) \} \div 150 \text{ mm}] + 1$
- = $[\{ 1900 \text{ mm} \} \div 150 \text{ mm}] + 1$
- = $12.67 + 1$
- = $13.67 = 14 \text{ nos.}$
- (By rounding off)

□ No. of bars along y-direction

- = $[\{ (\text{footing width}) - (2 \times \text{cover}) \} \div \text{spacing}] + 1$
- = $[\{ (1500 \text{ mm}) - (2 \times 50 \text{ mm}) \} \div 150 \text{ mm.}] + 1$
- = $[\{ 1400 \text{ mm} \} \div 150 \text{ mm}] + 1$
- = $9.33 + 1$
- = 10 nos.

□ Cutting length of the bar along the x-direction

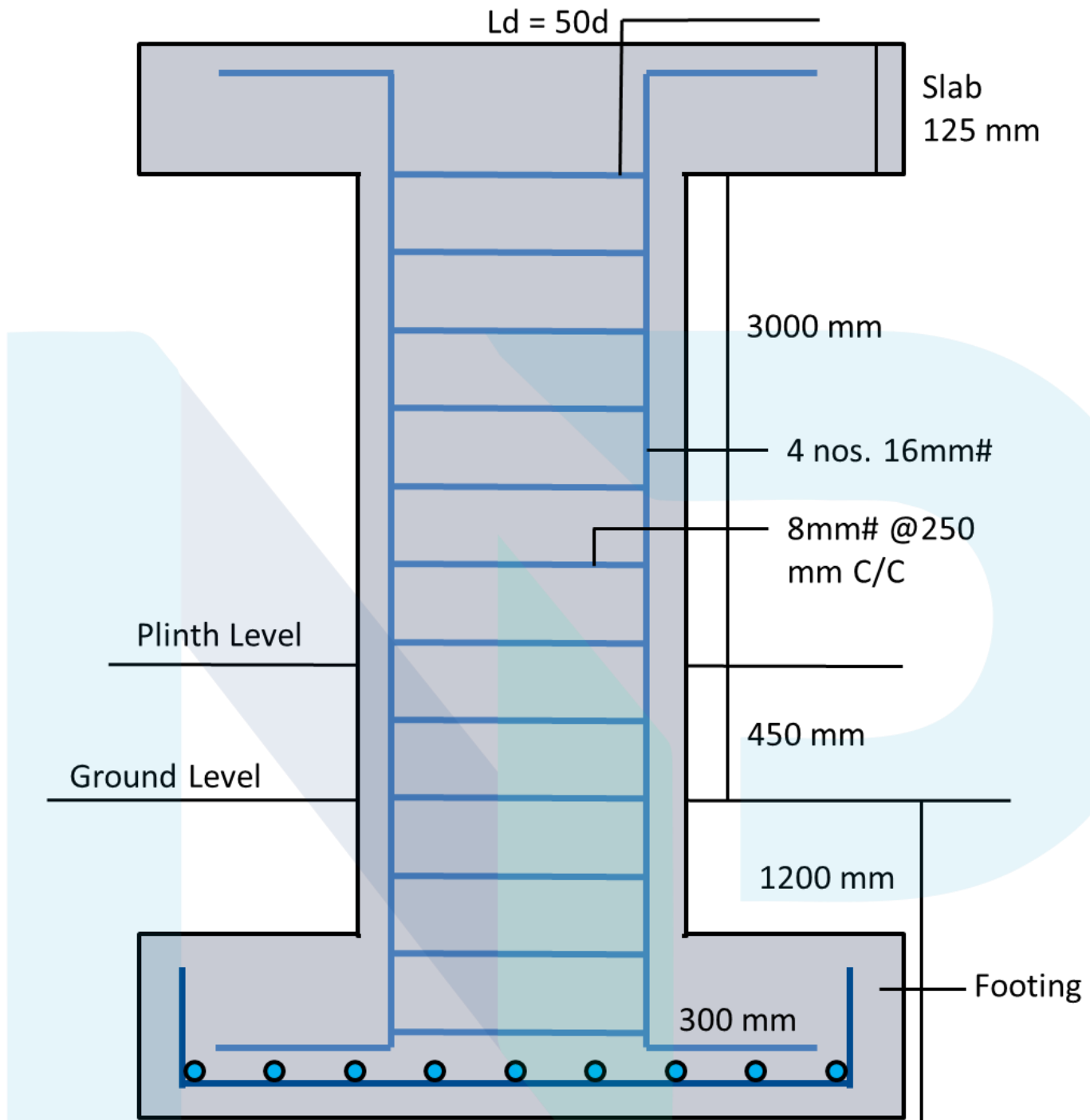
- = $[\{ \text{bar length in x-axis} \} + \{ 2 \text{ nos.} \times (L\text{- bend length}) \}] - 2 \text{ nos.} \times (2 \text{ times bar dia. for } 90^\circ \text{ bend.})$
- = $[\{ \text{footing length} - 2 \times \text{cover} \} + 2 \text{ nos.} \times \{ \text{footing height} - 2 \times \text{cover} \}] - 2 \times (2 \times \text{bar dia.})$
- = $[\{ 2000 \text{ mm} - 2 \times 50 \text{ mm} \} + 2 \times \{ 300 \text{ mm} - 2 \times 50 \text{ mm} \}] - 2 \times (2 \times 12 \text{ mm})$
- = $[1900 \text{ mm} + 400 \text{ mm}] - 48 \text{ mm}$
- = $2300 \text{ mm} - 48 \text{ mm}$
- = $2252 \text{ mm. i.e., } 2.252 \text{ m}$

□ Cutting length of the bar along the y-direction

- = $[\{ \text{bar length in y-axis} \} + \{ 2 \text{ nos.} \times (L\text{- bend length}) \}] - 2 \text{ nos.} \times (2 \text{ times bar dia. for } 90^\circ \text{ bend.})$
- = $[\{ \text{footing width} - 2 \times \text{cover} \} + 2 \text{ nos.} \times \{ \text{footing height} - 2 \times \text{cover} \}] - 2 \times (2 \times \text{bar dia.})$
- = $[\{ 1500 \text{ mm} - 2 \times 50 \text{ mm} \} + 2 \times \{ 300 \text{ mm} - 2 \times 50 \text{ mm} \}] - 2 \times (2 \times 12 \text{ mm})$
- = $[1400 \text{ mm} + 400 \text{ mm}] - 48 \text{ mm}$
- = $1800 \text{ mm} - 48 \text{ mm}$
- = $1752 \text{ mm. i.e. } 1.752 \text{ m.}$

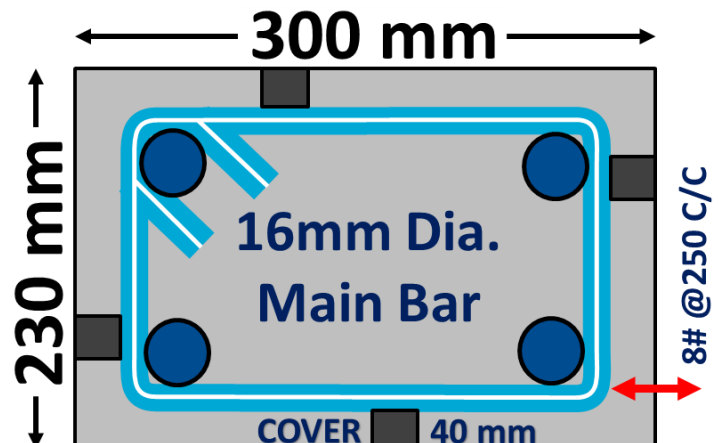
Column

Bar Bending Schedule of Column



Given data :

- Longitudinal bar dia. $d = 16 \text{ mm}$
- no. of bars = 4 no.
- Lateral ties bar dia. $d_1 = 8 \text{ mm}$
- spacing = 250 mm
- cover = 40 mm.
- Column size $x = 300 \text{ mm}$ & $y = 230 \text{ mm}$.
- Development length $L_d = 50d$



Bar Bending Schedule of Column

□ Length of the longitudinal bar

➤ = up to ground level + GL to plinth level + plinth level to slab bottom + slab cover + L_d + L-bend in footing - distance from footing bottom.

➤ = $\{1200 \text{ mm} + 450 \text{ mm} + 3000 \text{ mm} + 20 \text{ mm} + 50d + 300 \text{ mm}\} - 70 \text{ mm}$.

➤ = $\{4670 + (50 \times 16\text{mm}) + 300 \text{ mm}\} - 70 \text{ mm}$

➤ = $5770 \text{ mm} - 70 \text{ mm}$

➤ = 5700 mm i.e., 5.70 m



□ Length of the lateral ties

➤ = perimeter of lateral ties + total hook length - no. of bends

➤ = $2\text{sides} \times (x - 2 \times \text{cover}) + 2 \text{ sides} \times (y - 2 \times \text{cover}) + (2\text{nos} \times \text{hook length}) - (3 \text{ nos.} \times \text{bend})$

(Here, we have taken hook length = $12d_1$ for 135° & bend = $2d_1$ for 90°)

➤ = $\{[2 \times (300\text{mm} - 2 \times 40\text{mm})] + [2 \times (230 \text{ mm} - 2 \times 40 \text{ mm})]\} + \{2 \times 12 \times 8\text{mm}\} - \{3 \times 2 \times 8\text{mm}\}$

➤ = $\{[2 \times 220 \text{ mm}] + [2 \times 150 \text{ mm}]\} + 192 \text{ mm} - 48 \text{ mm}$

➤ = $\{440 \text{ mm} + 300 \text{ mm}\} + 144 \text{ mm}$

➤ = 884 mm i.e., 0.884 m .



□ Total number of lateral ties (stirrups)

➤ = $\{[\text{length of the longitudinal bar} - (L_d + L \text{ bend over footing})] \div \text{stirrup spacing}\} + 1$

➤ = $\{[5700 \text{ mm} - (50 \times 16 \text{ mm} + 300 \text{ mm})] \div 250 \text{ mm}\} + 1$

➤ = $\{[5700\text{mm} - 1100\text{mm}] \div 250 \text{ mm}\} + 1$

➤ = $\{4600 \text{ mm} \div 250 \text{ mm}\} + 1$

➤ = $18.4 + 1$

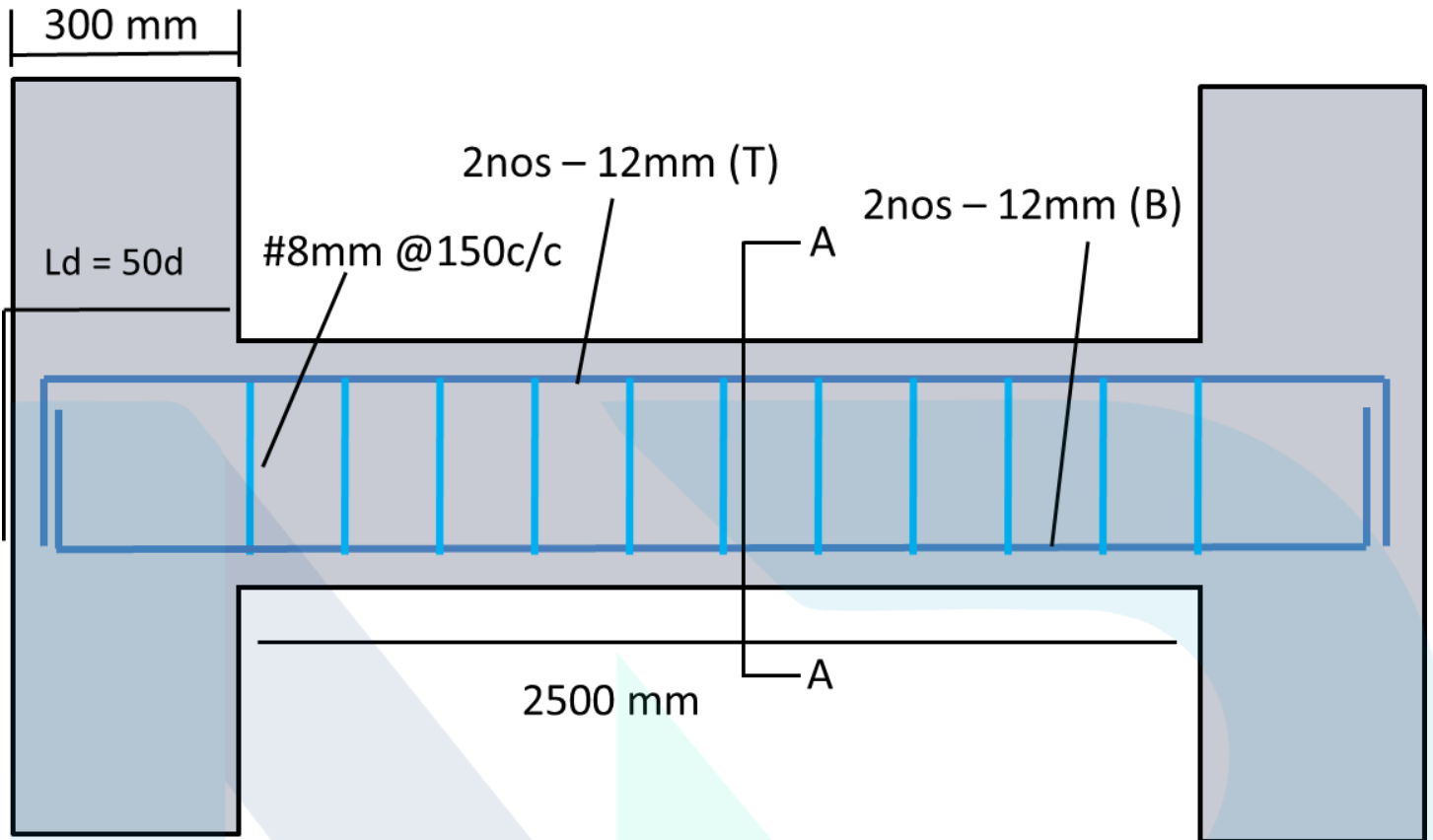
➤ = 19.4 nos.

➤ Rounding off, the number of stirrups required = 20 nos.

Note: $L_d + L$ bend is deducted from the length as no stirrups are provided over that length.

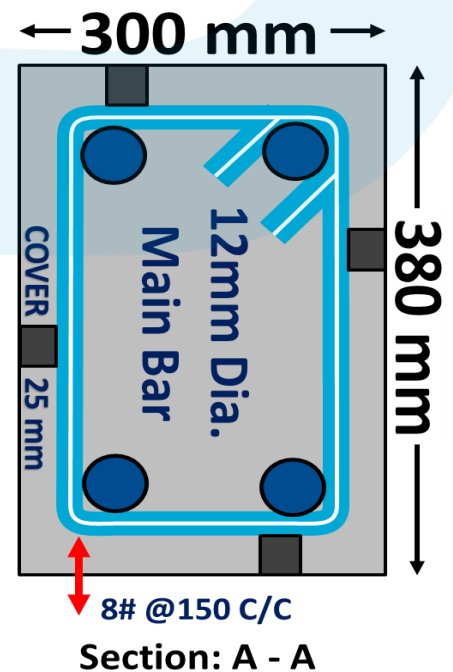
Beam

Bar Bending Schedule of Beam



Given data:

- Clear span of the beam = 2500 mm,
- Beam size = 300mm x 380 mm
- Top & bottom rebar dia. = 12 mm,
- Number of bars = 4 nos.
- Stirrups #8 mm @ 150 mm c/c
- Clear cover = 25 mm on all the sides.



The cutting length of the main bar

- = $[(2 \text{ times} \times Ld) + \text{inner distance between columns} - (2 \text{ nos.} \times 2d)]$
- (Let us take development length Ld as $50d$, and we have taken $2d$ for the 90° bend deductions)
- = $[(2 \times 50 \times 12 \text{ mm}) + 2500 \text{ mm} - (2 \text{ nos.} \times 2 \times 12 \text{ mm})]$
- = $[1200 \text{ mm} + 2500 \text{ mm} - 48 \text{ mm}]$
- = 3652 mm i.e., 3.652 m

Bar Bending Schedule of Beam

❑ Length of Lateral Ties

$$\Rightarrow = [2 \text{ nos.} \times \{(a + b)\} + (\text{hook length}) - (90^\circ \text{ bend}) - (135^\circ \text{ bend})]$$

Where,

$$\Rightarrow a = \text{beam width} - 2 \times \text{cover}$$

$$\Rightarrow b = \text{beam depth} - 2 \times \text{cover}$$

$$\Rightarrow = [2 \text{ nos.} \times \{(300 \text{ mm} - 2 \times 25 \text{ mm}) + (380 \text{ mm} - 2 \times 25 \text{ mm})\} + (2 \text{ nos.} \times 12d) - (3 \text{ nos.} \times 2d) - (2 \text{ nos.} \times 3d)]$$

(Here, 12d is taken for hook length.

We have deducted 2d for 90° bend -3nos., & 3d for 135° bend -2nos. as shown in the above drawing.)

$$\Rightarrow = [2 \text{ nos.} \times \{(250 \text{ mm}) + (330 \text{ mm})\} + (2 \text{ nos.} \times 12 \times 8 \text{ mm}) - (3 \text{ nos.} \times 2 \times 8 \text{ mm}) - (2 \text{ nos.} \times 3 \times 8 \text{ mm})]$$

$$\Rightarrow = [2 \text{ nos.} \times \{580 \text{ mm}\} + 192 \text{ mm} - 48 \text{ mm} - 48 \text{ mm}]$$

$$\Rightarrow = [1160 \text{ mm} + 192 \text{ mm} - 96 \text{ mm}]$$

$$\Rightarrow = 1256 \text{ mm i.e. } 1.256 \text{ m}$$



❑ Number of stirrups

$$\Rightarrow = (\text{plinth beam length} \div \text{stirrup spacing}) + 1$$

$$\Rightarrow = (2500 \text{ mm} \div 150 \text{ mm}) + 1$$

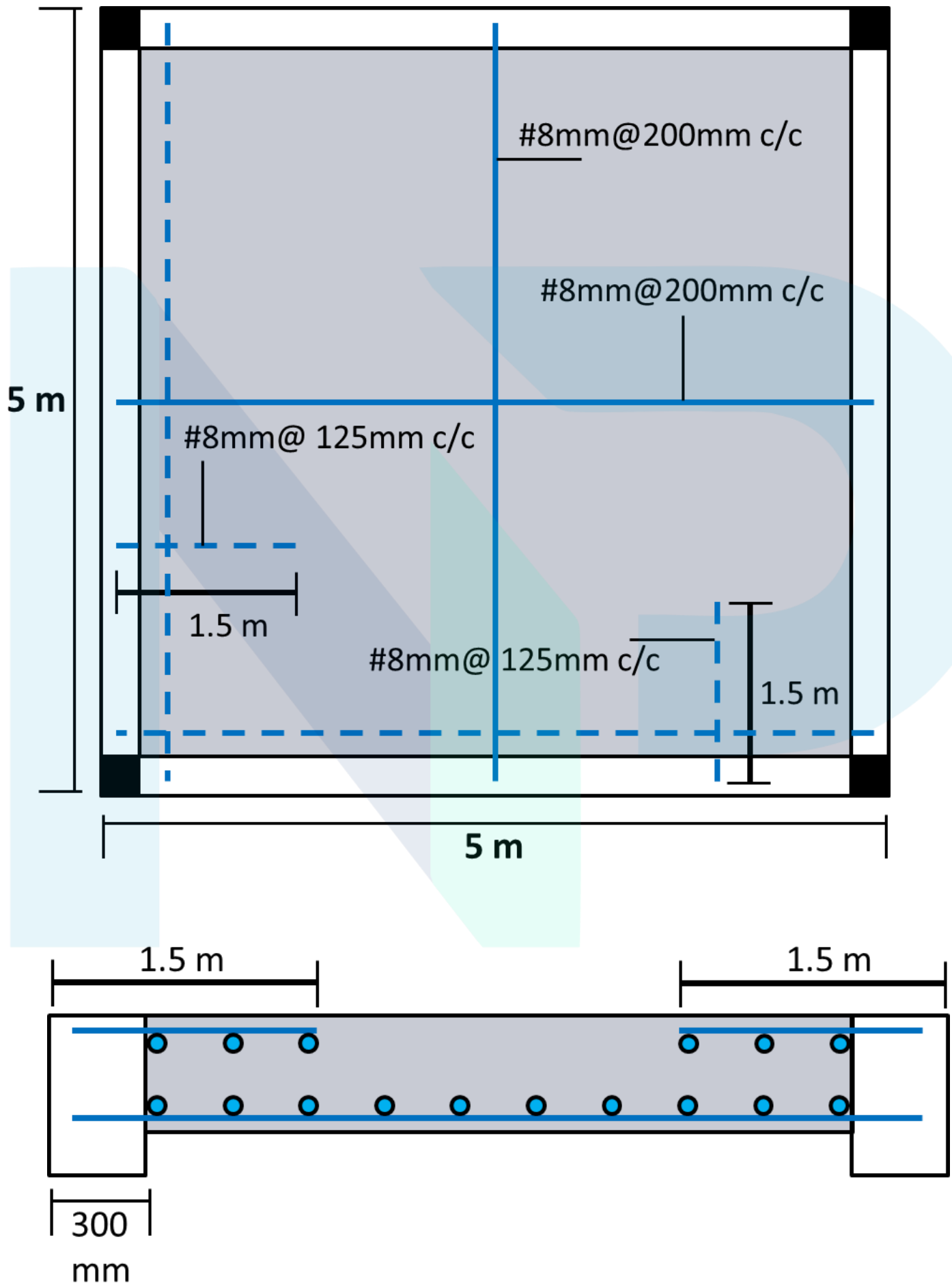
$$\Rightarrow = 16.66 + 1$$

$$\Rightarrow = 17.66 \text{ nos.}$$

$$\Rightarrow = \text{By rounding off, the no. of stirrups required} = 18 \text{ nos.}$$

Slab

Bar Bending Schedule of Slab



Bar Bending Schedule of Slab

☐ Details from the diagram:

- Thickness of slab is 150 mm.
- Bottom bar of slab will be provided 8 mm dia of steel is at 200 mm center to center spacing.
- Top bar of slab will be provided 8 mm dia of steel is at 200 mm center to center spacing.
- Extra bar of slab at x-axis will be provided 8 mm dia of steel is at 125 mm center to center spacing.
- Extra bar of slab at y-axis will be provided 8 mm dia of steel is at 125 mm center to center spacing.
- Top Distribution bar of slab at x-axis will be provided 8 mm dia of steel is at 125 mm center to center spacing.
- Top Distribution bar of slab at y-axis will be provided 8 mm dia of steel is at 125 mm center to center spacing.
- Concrete Cover of slab is 20 mm

☐ Steps for BBS of Slab:

☐ Numbers of steel in Main bar:

- Number of steel bar = $((\text{Total length of slab} - \text{Beam Width}) \div \text{spacing of bar}) + 1$
- Number of steel bar = $(5000 - 300) \div 200 + 1$
- Number of steel bar = 25 numbers

☐ Numbers of steel in Distribution bar:

- Number of steel bar = $((\text{Total length of slab} - \text{Beam Width}) \div \text{spacing of bar}) + 1$
- Number of steel bar = $(5000 - 300) \div 200 + 1$
- Number of steel bar = 25 numbers

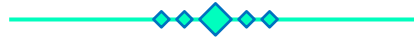
☐ Numbers of Extra bar at x-axis:

- Number of steels = $((\text{Total length of slab} - \text{Beam Width}) \div \text{spacing of bar}) + 1$
- Number of steel bar = $(5000 - 300) \div 125 + 1$
- Number of steel bar = 39 numbers
- As the Extra bars on both the X-axis side = $39 \times 2 = 78$

Bar Bending Schedule of Slab

❑ Numbers of Extra bar at y-axis:

- Number of steels = (Total length of slab ÷ spacing of bar) +1
- Number of steel bar = $(5000 - 300) \div 125 + 1$
- Number of steel bar = 39 numbers
- As the Extra bars on both the Y-axis side = $39 \times 2 = 78$



❑ Numbers of Top Distribution bar at x-axis:

- Number of steels = $((1500 - \text{Beam Width}) \div \text{spacing of bar}) + 1$
- Number of steel bar = $(1500 - 300) \div 125 + 1$
- Number of steel bar = 11 numbers
- As the Extra bars on both the X-axis side = $11 \times 2 = 22$



❑ Numbers of Top Distribution bar at y-axis:

- Number of steels = $((1500 - \text{Beam Width}) \div \text{spacing of bar}) + 1$
- Number of steel bar = $(1500 - 300) \div 125 + 1$
- Number of steel bar = 11 numbers
- As the Extra bars on both the Y-axis side = $11 \times 2 = 22$



❑ Cut Length of Main Bar:

- Length of Main bar = slab length - 2(concrete cover)
- = $5000 - 2(20)$
- = 4960 mm or 4.96 m
- Now we find the length of bar.
- The total length of Main bar = 4960 x number of steels in slab
- The total length of bar = 4960×25
- The total length of bar = 1,24,000 mm



❑ Cut Length of Distribution Bar:

- Length of Distribution bar = slab length - 2(concrete cover)
- = $5000 - 2(20)$
- = 4960 mm or 4.96 m
- Now we find the length of bar.
- The total length of Distribution Bar = 4960 x number of steels in slab
- The total length of Distribution Bar = 4960×25
- The total length of bar = 1,24,000 mm

Bar Bending Schedule of Slab

❑ Cut Length of Extra Bar at x-axis:

- Total Length of extra bar = Length of extra bar - concrete cover
- = $1500 - 20$
- = 1480 mm or 1.48 meter
- Now we find the length of bar.
- The total length of extra bar at x-axis = $1480 \times \text{number of extra bar in slab}$
- The total length of bar = 1480×78
- The total length of bar = 1,15,440 mm



❑ Cut Length of Extra Bar at y-axis:

- Length of Extra Bar at y-axis = slab length - concrete cover
- = $1500 - 20$
- = 1480 mm or 1.48 m
- Now we find the length of bar.
- The total length of Extra bar = $1480 \times \text{number of extra bar in slab}$
- The total length of bar = 1480×78
- The total length of bar = 1,15,440 mm



❑ Cut Length of Top Distribution Bar at x-axis:

- Length of Top Distribution bar = slab length - 2(concrete cover)
- = $5000 - 2(20)$
- = 4960 mm or 4.96 m
- Now we find the length of bar.
- The total length of Top Distribution Bar = $4960 \times \text{number of steels in slab}$
- The total length of Top Distribution Bar = 4960×25
- The total length of bar = 1,24,000 mm

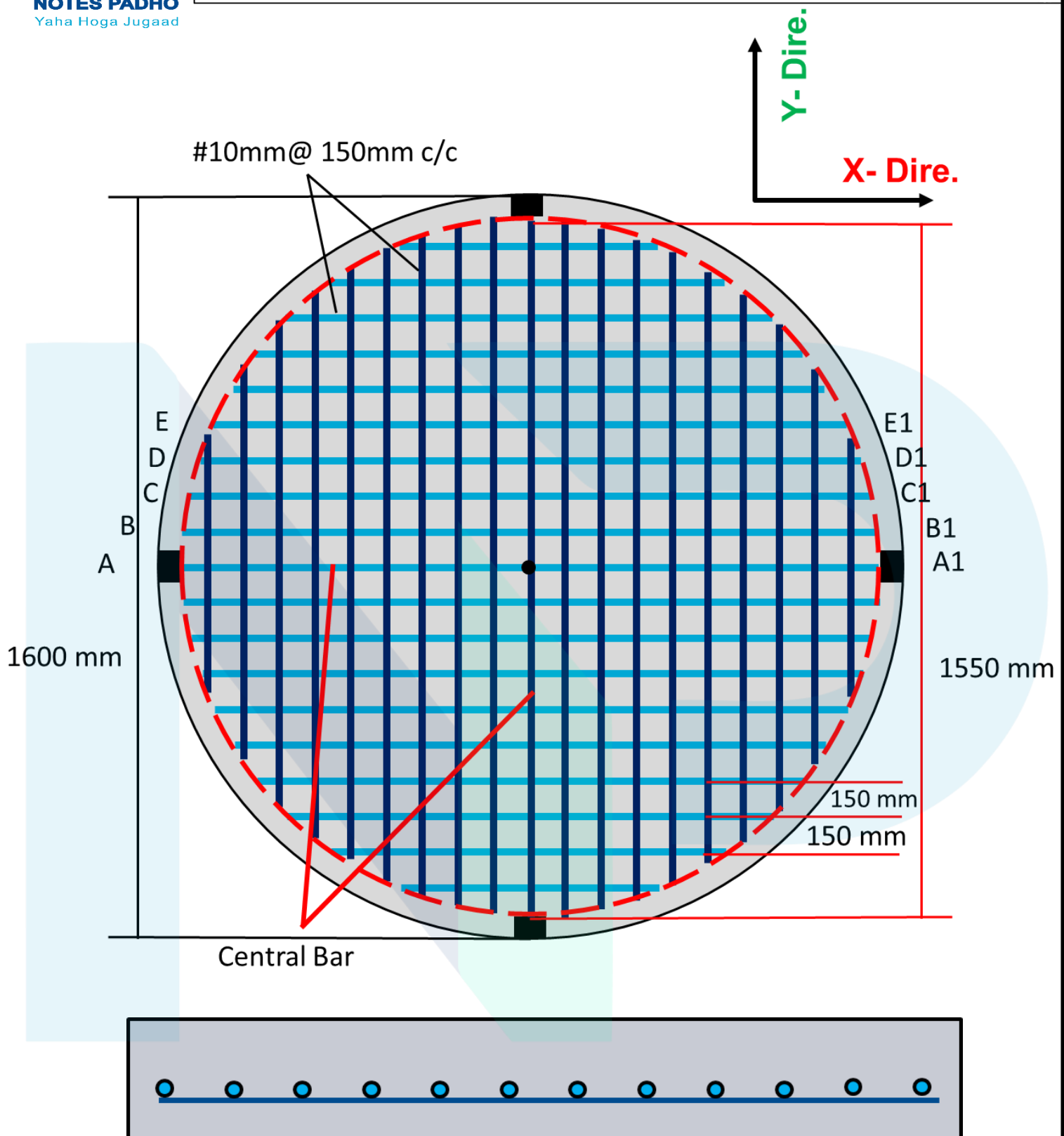


❑ Cut Length of Top Distribution Bar at y-axis:

- Length of Top Distribution bar = slab length - 2(concrete cover)
- = $5000 - 2(20)$
- = 4960 mm or 4.96 m
- Now we find the length of bar.
- The total length of Top Distribution Bar = $4960 \times \text{number of steels in slab}$
- The total length of Top Distribution Bar = 4960×25
- The total length of bar = 1,24,000 mm

Circular Slab

Bar Bending Schedule of Circular Slab



Given data:

- Diameter of the circular slab = 1600mm = 1.6m.
- Clear cover for the reinforcement = 25mm.
- Rebar diameter = 10mm.
- Rebar spacing = 150mm c/c

Bar Bending Schedule of Circular Slab

- ❑ The formula for finding the length of the bar
- $= 2\sqrt{r^2 - d^2}$

Where,

- $r = [\text{diameter of the slab} - (2 \text{ nos.} \times \text{clear cover})] \div 2$
- $d = \text{c/c distance of the individual bars from the central bar of the circle.}$
- As you can observe in the above-given drawing, I have drawn a red-coloured circle by deducting the clear cover. Radius r in the above formula will be the radius of this red circle.
- $r = [1600\text{mm} - (2 \text{ nos.} \times 25\text{mm})] \div 2$
- $= [1550\text{mm}] \div 2$
- $= 775\text{mm.}$
- The value of r will be the same for the cutting length calculation of every individual bar of the slab.
- **Note: All the rebars of the circular slab act as a chord of the red circle. To gain a basic understanding of this theory, you must go through the article,**
- **"The length of the individual bars in the upper half part of the circular slab will be different.**
- **The cutting length of the bars in the lower half portion will be equal to the upper half part bars.**
- **I have named the bars for the half portion of the Circular slab and let us find out the cutting length of each of these bars."**



- ❑ 1. Cutting length of bar AA1
- $= \text{diameter of red circle} = 1550\text{mm}$



- ❑ 2. Cutting length of bar BB1
- $= 2\sqrt{r^2 - d^2}$
- Here, $d = 150\text{mm.}$
- $= 2 \times \sqrt{775^2 - 150^2}$
- $= 2 \times \sqrt{600625 - 22500}$
- $= 2 \times \sqrt{578125}$
- $= 2 \times 760.345$
- $= 1520.69\text{mm} = 1.521\text{m}$

❑ 3. Cutting length of the bar CC1

- Here, $d = 300\text{mm}$.
- $= 2\sqrt{r^2 - d^2}$
- $= 2 \times \sqrt{7752 - 3002}$
- $= 2 \times \sqrt{600625 - 90000}$
- $= 2 \times \sqrt{510625}$
- $= 2 \times 714.58$
- $= 1429.16\text{mm} = 1.429\text{m}$

❑ 4. Cutting length of the bar DD1

- Here, $d = 450\text{mm}$.
- $= 2\sqrt{r^2 - d^2}$
- $= 2 \times \sqrt{7752 - 4502}$
- $= 2 \times \sqrt{600625 - 202500}$
- $= 2 \times \sqrt{398125}$
- $= 2 \times 630.971$
- $= 1261.94\text{mm} = 1.261\text{m}$

❑ 5. Cutting length of the bar EE1

- Here, $d = 600\text{mm}$.
- $= 2\sqrt{r^2 - d^2}$
- $= 2 \times \sqrt{7752 - 6002}$
- $= 2 \times \sqrt{600625 - 360000}$
- $= 2 \times \sqrt{240625}$
- $= 2 \times 490.535$
- $= 981.07\text{mm} = 0.981\text{m}$

❑ 6. Cutting length of the bar FF1

- Here, $d = 750\text{mm}$.
- $= 2\sqrt{r^2 - d^2}$
- $= 2 \times \sqrt{7752 - 7502}$
- $= 2 \times \sqrt{600625 - 562500}$
- $= 2 \times \sqrt{38125}$
- $= 2 \times 195.256$
- $= 390.51\text{mm} = 0.39\text{ m}$

Bar Bending Schedule of Circular Slab

- ❑ No. of bars in a circular slab
 - The no. of top bars in the circular slab
 - = $[(\text{diameter of the slab}) \div \text{c/c bar spacing}] + 1$
 - = $[(1600\text{mm}) \div 150\text{mm}] + 1$
 - = 11 nos.



- ❑ That means 5 bars on either side of the central bar AA1
 - Total no. of bars
 - = Top bars + Bottom bars
 - = 11nos + 11nos.=22nos.



- ❑ As the upper half part of the circle is identical to the bottom half part, you will have 2nos of central common bars and 4 nos. of the chord bars having the same cutting length.

1. Cutting length of central bar AA1 = 1.55m = 2nos.
2. Cutting length of bar BB1 = 1.521m = 4nos.
3. Cutting length of bar CC1 = 1.429m = 4nos.
4. Cutting length of bar DD1 = 1.261m = 4nos.
5. Cutting length of bar EE1 = 0.981m = 4nos.

BBS Excel Format

Bar Bending Schedule Format

Serial No. Details of element you are solving

Number of element you are solving

Total length of bar in element

Cut Length of Element

No. Of Bar

Total Wt. of Bar

S.No.	Description	Shape of Bar	No. of Items	Dia of bar	No. of Bars	CL ₁ (m)	CL ₂ (m)	CL ₃ (m)	Cutting Length (m)	Total Length	Unit Wt.	Total Wt.	8mm (KG)	10mm (KG)	12mm (KG)	16 mm (kg)
FOOTING																
1	F1															
	X-dir	200 	15	12 #	14				2.25	472.92	0.889	420.37	.	.	420.37	.
	Y-dir	200 		12 #	10				1.75	262.80	0.889	233.60	.	.	233.60	.
2	F2															
	X-dir	200 	8	12 #	12				1.75	168.19	0.889	149.50	.	.	149.50	.
	Y-dir	200 		12 #	8				1.28	81.92	0.889	72.82	.	.	72.82	.

Bar shape req. for bar bender

Dia. Of Bar

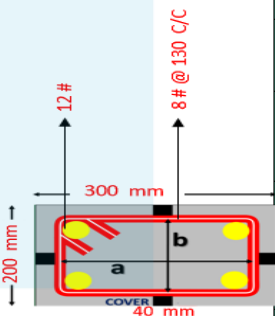
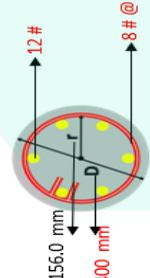
Diff. Cut Length value in one element ex. 1 column but different cut length after lap.

Unit Wt. of Bar directly put formula

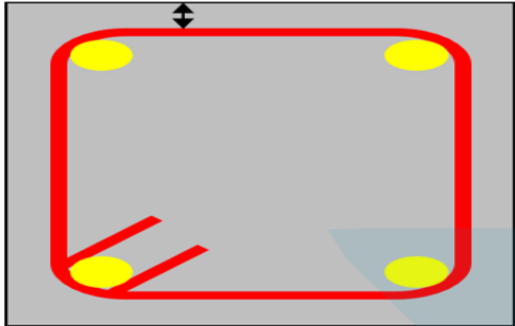
Put value of Total wt. under same dia. of bar. Helps to calculate quantity at end.

Visit : www.NotesPadho.com to Download This Excel Format

Bar Bending Schedule Automatic Format for Stirrup

BBS of Rectangular Stirrup													
General Data													
Section	200 mm	300 mm											
Main Dia at corner	12 #												
Dia of tie/ Stirrups	8 # @ 130 C/C												
Clear Cover	40 mm												
													
S.No.	Description	Shape of Bar	No. of Items	Dia of bar	No. of bars	Cutting length (m)	Total Length	Unit Wt	Total Wt.	8mm (KG)	10mm (KG)	12mm (KG)	16 mm(kg)
	2LG.ST		1	8 #	1	0.84 m	0.84 m	0.40 kg	0.33 kg	0.33 kg	-	-	-
BBS of Circular Stirrup													
General Data													
Circular Column Dia	400 mm												
Main Dia at corner	12 #												
Dia of tie/ Stirrups	8 # @ 130 C/C												
Clear Cover	40 mm												
													
				$r (= D/2 - \text{Cover} - \text{Half dia. of})$									
				156.0 mm									
S.No.	Description	Shape of Bar	No. of Items	Dia of bar	No. of bars	Cutting length (m)	Total Length	Unit Wt	Total Wt.	8mm (KG)	10mm (KG)	12mm (KG)	16 mm(kg)
	2LG.ST		1	8 #	1	1.14 m	1.14 m	0.40 kg	0.45 kg	0.45 kg	-	-	-

Section	Size of the Column or Beam
Main Dia.	Diameter of Column or Beam reinforcement.
Dia. of Tie	Diameter Of Stirrups along with c/c spacing between them.
Clear Cover	The clear cover is the distance between the exposed concrete surface to



In general / min
Beam - 25 mm
Column -40 mm

CLEAR

COVER

In general / min
Beam - 25 mm
Column -40 mm

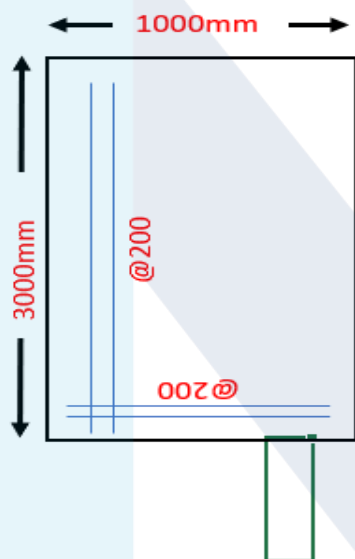
Note: This automatic excel format will help you calculate BBS of different components. But you should learn manual calculation first because automatic calculation is not always 100% accurate. Value changes as per different conditions.

Visit : www.NotesPadho.com to Download This Excel Format

Bar Bending Schedule Automatic Format for Footing

BBS of Double mesh Footing

General Data	Footing ID	F2
	Footing Size	1000mm 3000mm
	Height (H)	800mm
		200mm
	Y -dir Top Reinforcement	16# @200C/C
	X -dir Top Reinforcement	16# @200C/C
	Y -dir Bottom Reinforcement	16# @150C/C
	X -dir Bottom Reinforcement	16# @150C/C
	Clear Cover	50mm
	No. of Items	1



S.No.	Description	Shape of Bar	No. of Items	Dia of bar	No. of bars	Cutting length (m)	Total Length	Unit Wt	Total Wt.	8mm (KG)	10mm (KG)	12mm (KG)	16 mm(kg)
	Footing F2												
	X -dir Top Reinforcement		1	16#	16	1.94	30.98 m	1.58 kg	48.95 kg	-	-	-	48.95 kg
	Y -dir Top Reinforcement		1	16#	6	3.90	23.42 m	1.58 kg	37.02 kg	-	-	-	37.02 kg
	X -dir Bottom Reinforcement		1	16#	21	1.94	40.66 m	1.58 kg	64.25 kg	-	-	-	64.25 kg
	Y -dir Bottom Reinforcement		1	16#	7	3.90	27.33 m	1.58 kg	43.18 kg	-	-	-	43.18 kg

Note: This automatic excel format will help you calculate BBS of different components. But you should learn manual calculation first because automatic calculation is not always 100% accurate. Value changes as per different conditions.

Visit : www.NotesPadho.com to Download This Excel Format

Column BBS

Description	value
UG Pedestal Depth	1700
Footing Depth	300
Plinth Beam Depth	300
Floor Beam Depth	300
No of Floor Beam	1
Floor Height	3000
Parapet Wall Height	500
Footing Development Length	300
Dia Of Bar	12
Cover	40
Footing Main Bar	12
Footing Distribution Bar	12
No of Bar in Column	6

Description	Value
Cutting Length Of One Bar	6336
Weight Of One Bar	5.632
Steel Required for 1 Column	38.016
Steel Weight for 1 Column	33.792

Note: This automatic excel format will help you calculate BBS of different components. But you should learn manual calculation first because automatic calculation is not always 100% accurate. Value changes as per different conditions.

Visit : www.NotesPadho.com to Download This Excel Format

Bar Bending Schedule of Beam

General Data					
Beam ID		B2			
First Support		Beam	250 mm	350 mm	
Last Support		Beam	250 mm	350 mm	
					@120C/C
				8#	300 mm
				250 mm	
				Development Length	50 d
				Lap Length	50 d
Support					
Members (C/C)		Member 1	2395 mm	Intermediate Support 1	250 mm
		Member 2	3050 mm	Intermediate Support 2	250 mm
		Member 3	3255 mm	Intermediate Support 3	250 mm
		Member 4	2375 mm	Intermediate Support 4	400 mm
		Member 5	2510 mm	Intermediate Support 5	400 mm
		Member 6	2375 mm	Intermediate Support 6	250 mm
		Member 7	3255 mm	Intermediate Support 7	250 mm
		Member 8	3050 mm	Intermediate Support 8	250 mm
		Member 9	2395 mm	Intermediate Support 9	
		Member 10			
Main Reinforcement					
			Top	2	12
			Bottom	2	12

Note: This automatic excel format will help you calculate BBS of different components. But you should learn manual calculation first because automatic calculation is not always 100% accurate. Value changes as per different conditions.

Visit : www.NotesPadho.com to Download This Excel Format

Bar Bending Schedule Automatic Format for Slab

BBS of Slab

S.NO.	SIZE OF SLAB	LONGER SPAN (LY)	SHORTER SPAN (LX)	COVER	THICKNESS OF SLAB	MAIN BAR	VOLUME OF H DISTRIBUTION BAR
1	TYPE OF BAR	6000	4000	20	125	77	77
2	MAIN BAR	DIA OF BAR	SPACING				
3	DISTRIBUTION BAR	8	130				
4	TOP EXTRA BAR IN LEFT & RIGHT	8	130				
5	TOP EXTRA BAR IN TOP & BOTTOM SIDE	8	260				
6	TOP EXTRA DISTRIBUTION BAR IN LEFT & RIGHT SIDE	8	130				
7	TOP EXTRA DISTRIBUTION BAR IN TOP & BOTTOM SIDE	8	130				

S.NO.	TYPES OF BAR	NO. OF BAR	DIA. OF BAR(IN MM)	CUTTING LENGTH(IN M)	TOTAL LENGTH(IN M)	UNIT WT.	TOTAL WT(IN KG)	Unit
1	MAIN BAR	48	8	4.784	229.64832	0.395	90.73	
2	DISTRIBUTION BAR	32	8	6.784	217.09888	0.395	85.77	
3	TOP EXTRA BAR IN LEFT & RIGHT	18	8	1.717	30.912	0.395	12.21	
4	TOP EXTRA BAR IN TOP & BOTTOM SIDE	14	8	2.384	33.376	0.395	13.19	
5	TOP EXTRA DISTRIBUTION BAR IN LEFT & RIGHT SIDE	24	8	6.768	162.432	0.395	64.17	
6	TOP EXTRA DISTRIBUTION BAR IN TOP & BOTTOM SIDE	34	8	4.768	162.112	0.395	64.04	
TOTAL WT. OF STEEL TO BE USED IN TWO-WAY SLAB							330.11	Kg
ADD 5% WASTAGE							16.51	
GRAND TOTAL WT. OF TWO-WAY SLAB							346.61	Kg

Note: This automatic excel format will help you calculate BBS of different components. But you should learn manual calculation first because automatic calculation is not always 100% accurate. Value changes as per different conditions.

Visit : www.NotesPadho.com to Download This Excel Format

Size (outer to outer)		4000mm
Reinforcement	12#	#####
Clear cover	20mm	
No of items	1	

r 1980													
S.No.	DESCRIP TION	SHAPE OF BARS	NO. OF ITEMS	DIA.OF BARS	NO. OF BARS	CUTTING LENGTH	TOTAL LENGTH	UNIT WEIGHT	TOTAL WEIGHT	8mm(Kg)	10mm(Kg)	12mm(Kg)	16mm(Kg)
	L1		1	12 #	2	3.96 m	7.92 m	0.89 kg	7.04 kg	-	-	7.04 kg	-
	L2		1	12 #	4	3.95 m	15.79 m	0.89 kg	14.04 kg	-	-	14.04 kg	-
	L3		1	12 #	4	3.91 m	15.66 m	0.89 kg	13.92 kg	-	-	13.92 kg	-
	L4		1	12 #	4	3.86 m	15.43 m	0.89 kg	13.71 kg	-	-	13.71 kg	-
	L5		1	12 #	4	3.77 m	15.10 m	0.89 kg	13.42 kg	-	-	13.42 kg	-
	L6		1	12 #	4	3.66 m	14.66 m	0.89 kg	13.03 kg	-	-	13.03 kg	-
	L7		1	12 #	4	3.53 m	14.11 m	0.89 kg	12.54 kg	-	-	12.54 kg	-
	L8		1	12 #	4	3.36 m	13.43 m	0.89 kg	11.94 kg	-	-	11.94 kg	-
	L9		1	12 #	4	3.15 m	12.60 m	0.89 kg	11.20 kg	-	-	11.20 kg	-
	L10		1	12 #	4	2.90 m	11.59 m	0.89 kg	10.30 kg	-	-	10.30 kg	-
	L11		1	12 #	4	2.58 m	10.34 m	0.89 kg	9.19 kg	-	-	9.19 kg	-
	L12		1	12 #	4	2.19 m	8.76 m	0.89 kg	7.78 kg	-	-	7.78 kg	-
	L13		1	12 #	4	1.65 m	6.60 m	0.89 kg	5.87 kg	-	-	5.87 kg	-
	L14		1	12 #	4	0.69 m	2.75 m	0.89 kg	2.44 kg	-	-	2.44 kg	-

Note: This automatic excel format will help you calculate BBS of different components. But you should learn manual calculation first because automatic calculation is not always 100% accurate. Value changes as per different conditions.

Visit : www.NotesPadho.com to Download This Excel Format

Bar Bending Schedule Automatic Format for Sunken Slab

Bar Bending Schedule of Sunken Slab													
General Data	Sunken Slab	S4											
	Longer Span	2625 mm	Beam Section	250 mm	300 mm								
	Shorter Span	1025 mm	Clear Cover of Beam	25 mm									
			Top Dis at										
	Clear Cover of Slab	20 mm	Dia. Of Main Bar	8	125								
	No. of Items	4	Dia. Of Dis. Bar	8	125								
S.No.	Description	Shape of Bar	No. of Items	Dia of bar	No. of bars	Cutting lenngth (m)	Total Length	Unit Wt	Total Wt.	8mm (KG)	10mm (KG)	12mm (KG)	16 mm(KG)
	Sunken Slab S4												
	Bottom Main Bar		4	8 #	22	1.48	129.80 m	0.40 kg	51.28 kg	51.28 kg	-	-	-
	Bottom Distribution Bar		4	8 #	10	3.08	123.00 m	0.40 kg	48.59 kg	48.59 kg	-	-	-
	Top Main Bar		4	8 #	22	1.48	129.80 m	0.40 kg	51.28 kg	51.28 kg	-	-	-
	Top Distribution Bar		4	8 #	10	3.08	123.00 m	0.40 kg	48.59 kg	48.59 kg	-	-	-

Note: This automatic excel format will help you calculate BBS of different components. But you should learn manual calculation first because automatic calculation is not always 100% accurate. Value changes as per different conditions.

Visit : www.NotesPadho.com to Download This Excel Format

BBS Sample Drawings

Sample Drawing

PROJECT TITLE

Notes Padho

SPECIFICATIONS

- ANY DISCREPANCY FOUND BETWEEN ARCHITECTURAL DRAWING AND STRUCTURAL DRAWING SHOULD BE BROUGHT TO ARCHITECT'S NOTICE BEFORE EXECUTION.
- STEEL USED IS Fe 415.
- COVER TO STEEL (FOR COLUMN- 25 MM CLEAR AND FOR FOOTING- 40MM CLEAR).
- REFER TO THE WRITTEN DIMENSION ONLY.
- COLUMN C4 VARY FROM SCHEMATIC TO THE ANSWERS GIVEN BY EXAMINERS

S.No	COLUMN NAME	COLUMN SIZE (M)
1	C1	0.3X0.2
2	C2	0.45X0.2
3	C3	0.6X0.2
4	C4	0.3X0.2
5	C5	0.2X0.2

NOTES

- ALL DIMENSIONS ARE IN METRE.
- DETAIL FOR STRAP BEAM TO BE REFERRED FROM SHEET 2B.

DATE

NORTH

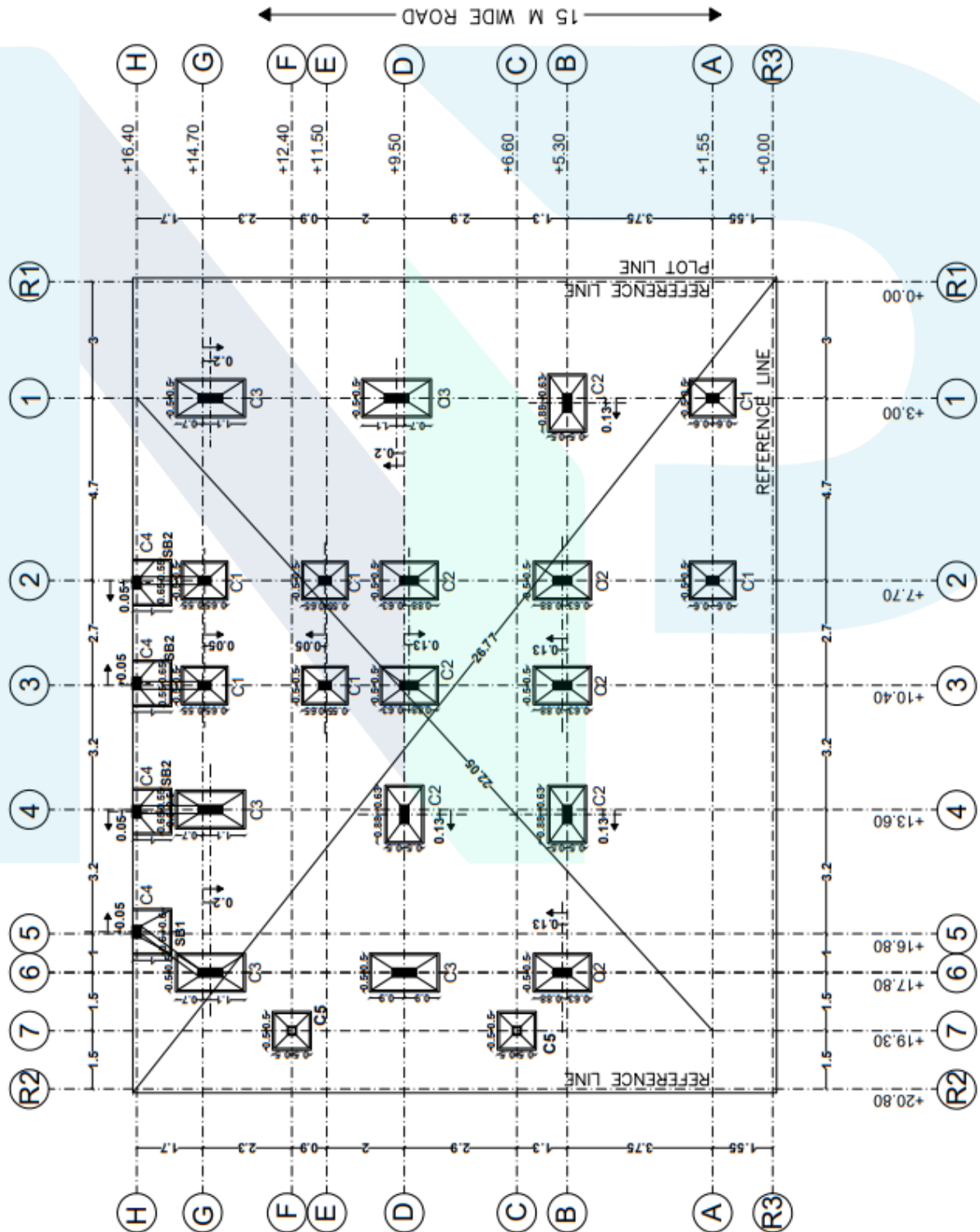


SCALE
1:100

SHEET NO
2A

SHEET TITLE
COLUMN FOOTING
AND EXCAVATION
PLAN

Educational Purpose
Only



PROJECT TITLE

Notes Padho

SPECIFICATIONS

- ANY DISCREPANCY FOUND BETWEEN ARCHITECTURAL DRAWING AND STRUCTURAL DRAWING SHOULD BE BROUGHT TO ARCHITECT'S NOTICE BEFORE EXECUTION.
- STEEL USED IS Fe 415.
- COVER TO STEEL (FOR COLUMN- 25 MM CLEAR AND FOR FOOTING- 40MM CLEAR).
- REFER TO THE WRITTEN DIMENSION ONLY.
- COLUMN C4 VARY FROM SCHEDULE TO THE DIMENSIONS SHOWN IN THE DRAWINGS.

S.NO	COLUMN NAME	COLUMN SIZE (M)
1	C1	0.3X0.2
2	C2	0.45X0.2
3	C3	0.6X0.2
4	C4	0.3X0.2
5	C5	0.2X0.2

NOTES

- ALL DIMENSIONS ARE IN METRE.
- DETAIL FOR STRAP BEAM TO BE REFERRED FROM SHEET 2B.

DATE

NORTH



SCALE

1:100

SHEET NO

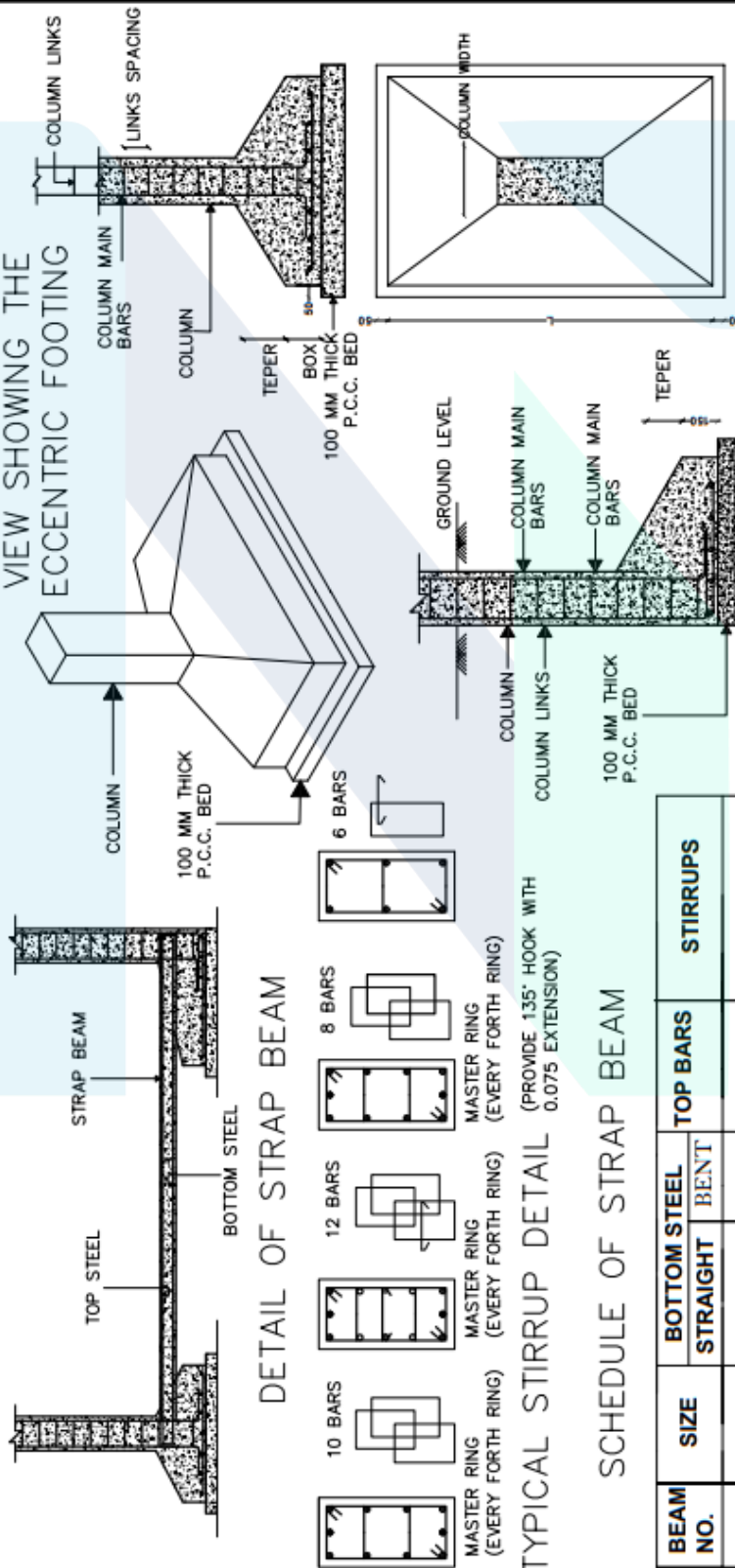
2B

SHEET TITLE

COLUMN FOOTING DETAILS

Educational Purpose Only

VIEW SHOWING THE ECCENTRIC FOOTING



DETAIL OF STRAP BEAM

TYPICAL STIRRUP DETAIL

SCHEDULE OF STRAP BEAM

BEAM NO.	SIZE	BOTTOM STEEL		TOP BARS		STIRRUPS	
		STRAIGHT	BENT	2 X 16	2 X 16	8 Ø	@ 150 C/C
SB	0.2x0.45	2 X 16	2 X 16	2 X 16	2 X 16	8 Ø	@ 150 C/C

TYPICAL DETAIL OF COLUMN FOOTING ECCENTRIC FOOTING COLUMN FOOTING

COL. EXCAVATION NO.	EXCAVATION SIZE (m)	P.C.C THICK (m)	FOOTING SIZE (m)	FOOTING DEPTH		REINFORCEMENT ALONG			COLUMN SIZE (m)	VERTICAL BARS	RINGS C/C	COLUMN SECTION AT G. FLOOR
				AT COLUMN FACE (m)	AT FOOTING END (m)	LONG LOWER TIER	SHORT UPPER TIER	SHORT TIER				
C1	1x1.2	0.1	0.9x1.1	0.38	0.15	7 X 10	8 X 10	4 X 12	0.2x0.3	4 X 12	8 Ø @ 150 C/C	
C2	1x1.5	0.1	0.9x1.4	0.45	0.15	7 X 12	8 X 12	4 X 12	0.2x0.45	4 X 12	8 Ø @ 150 C/C	
C3	1x1.8	0.1	0.9x1.7	0.6	0.15	7 X 12	9 X 12	10 X 12	0.2x0.6	10 X 12	8 Ø @ 150 C/C	
C4	0.6x1.2	0.1	0.9x1.1	0.4	0.15	7 X 10	8 X 10	6 X 12	0.2x0.3	6 X 12	8 Ø @ 150 C/C	
C5	1x1	0.1	0.9x0.9	0.4	0.15	6 X 10	8 X 10	4 X 12	0.2x0.2	4 X 12	8 Ø @ 150 C/C	

PROJECT TITLE
Notes Padho

SPECIFICATIONS
ANY DISCREPANCY FOUND BETWEEN ARCHITECTURAL DRAWING AND STRUCTURAL DRAWING SHOULD BE BROUGHT TO ARCHITECT'S NOTICE BEFORE EXECUTION. ALL CONCRETE MIX M-20 AND STEEL Fe-250 EXCEPT 6MM DIA. OF BARS. PLEASE READ THIS ARCHITECTURAL DRAWING FOR BEAM SLAB POSITION AND CANTILEVER PORTIONS. CONTRACTOR IS RESPONSIBLE FOR SUPERVISION OF COLUMNS

S.NO	COLUMN NAME	COLUMN SIZE (M)
1	C1	0.3X0.2
2	C2	0.45X0.2
3	C3	0.6X0.2
4	C4	0.3X0.2
5	C5	0.2X0.2

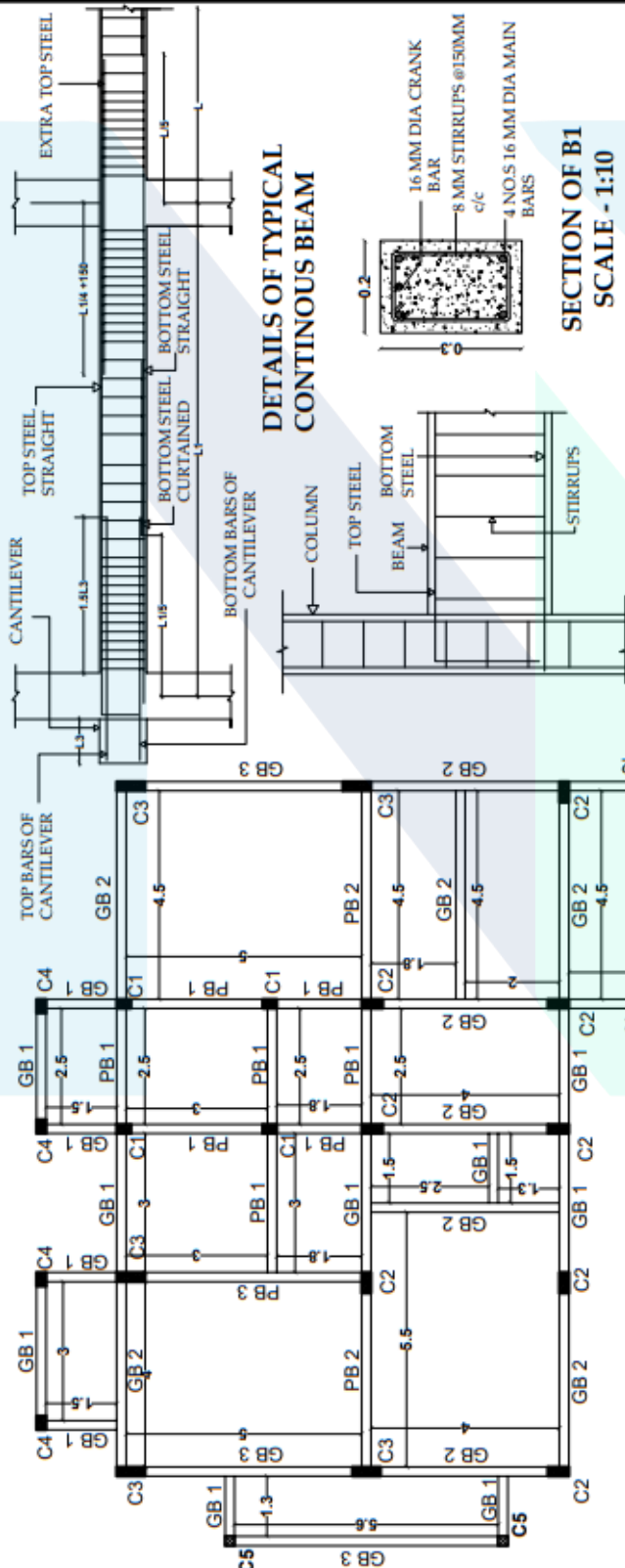
NOTES
ALL DIMENSIONS ARE IN METRE.

DATE
NORTH

SCALE
1:100

SHEET TITLE
GROUND BEAM AND PLINTH BEAM LAYOUT PLAN

Educational Purpose Only



KEY PLAN FOR GROUND BEAMS AND PLINTH BEAMS

NOTE: PB = PLINTH BEAM @ PLINTH LEVEL
GB = GROUND BEAM @ GROUND LEVEL

SCHEDULE OF BEAMS AT PLINTH LEVEL

BEAM NO.	SIZE/THICKNESS BXD (OVERALL) M	MID SPAN MAIN BAR (MM)		TOP BARS (MM)	RINGS / DISTRIBUTION C/C (MM)
		BOTTOM STRAIGHT BARS	BOTTOM B/UP AT 2D FROM FACE		
B1	2 X 3	2 X 12 Φ	1 X 10 Φ	2 X 10 Φ	8 Φ @ 150 C/C
B2	2 X 4.5	2 X 12 Φ	2 X 10 Φ	2 X 10 Φ	8 Φ @ 150 C/C
B3	2 X 6	2 X 12 Φ	2 X 12 Φ	2 X 10 Φ	8 Φ @ 150 C/C

NOTE:

- TENSION OVERLAP 60 X DIA. AND COMPRESSION OVERLAP 30 X DIA OF BARS.
- MINIMUM CLEAR COVER FOR
 - (a) SLAB = 15 MM
 - (b) BEAM = 25 MM
- PROVIDE 8 Φ @ 300 C/C 1200 LONG BARS AS EXTRA TOP OVER THOSE BEAMS WHICH ARE PARALLEL TO MAIN REINFORCEMENT OF SLAB.
- BENT UP BARS SHALL BE EXTENDED UP TO 0.3 L IN ADJACENT SPAN OVER CONTINUOUS SUPPORT IF EXTRA TOP NOT PROVIDED AND ANCHOR DOWN IN END SUPPORT.
- NOT MORE THAN HALF THE COLUMN BARS SHALL BE LAPPED AT THE SECTION.
- USE OF NEEDLE VIBRATOR AND PLASTICIZERS RECOMMENDED FOR R.C.C. WORK.
- USE DENSIFIED COATED PLYWOOD FOR SHUTTERING AND FORMWORK.
- USE STEEL PROPS AND STEEL SUPPORT FOR FORMWORK.
- BURNED OIL NOT PERMITTED FOR DE-SHUTTERING.
- CURING - EXPOSED SURFACE OF CONCRETE SHALL BE KEPT CONTINUOUSLY IN WET CONDITION FOR MINIMUM 14 DAYS.
- GHODI OR CHAIR HEIGHT = SLAB THICKNESS + (15MM + DIA OF SLAB MAIN BAR).

PROJECT TITLE
Notes Padho

SPECIFICATIONS
• ANY DISCREPANCY FOUND BETWEEN ARCHITECTURAL DRAWING AND STRUCTURAL DRAWING SHOULD BE BROUGHT TO ARCHITECT'S NOTICE BEFORE EXECUTION.
• ALL CONCRETE MIX 1:2:20 AND STEEL Fe-250 EXCEPT 6MM DIA. OF BARS.
• PLEASE READ THIS DRAWING WITH RELEVANT ARCHITECTURAL DRAWING FOR BEAM, SLAB POSITION AND CANTILEVER PORTIONS. CONTRACTOR IS RESPONSIBLE FOR SURRENDER OF COLUMNS

S.No	COLUMN NAME	COLUMN SIZE (M)
1	C1	0.3X0.2
2	C2	0.45X0.2
3	C3	0.6X0.2
4	C4	0.3X0.2
5	C5	0.2X0.2

NOTES
ALL DIMENSIONS ARE IN METRE.

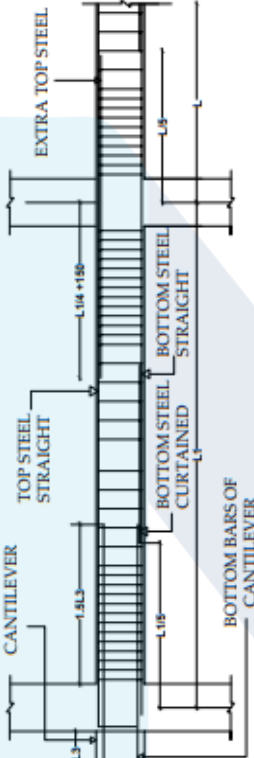
DATE
NORTH

SCALE
1:100

SHEET NO
06

SHEET TITLE
GROUND FLOOR SLAB DETAILS

Educational Purpose Only



DETAILS OF TYPICAL CONTINUOUS BEAM

SLAB TH. NO.	TYPE	MID SPAN STEEL	OTHER SPAN STEEL	REMARKS
S1	TWO WAY	# 8 @ 180 c/c ALT. BENT @ L/4	# 8 @ 200 c/c ALT. BENT @ L/4	-----
S2	ONE WAY	# 8 @ 180 c/c ALT. BENT @ L/4	# 6 @ 150 c/c	-----
S3	ONE WAY	# 8 @ 180 c/c ALT. BENT @ L/4	# 8 @ 180 c/c	SLAB FROM BEAM BOTTOM

DETAIL OF SLAB

NOTE:

- TENSION OVERLAP 60 X DIA. AND COMPRESSION OVERLAP 30 X DIA OF BARS.
- MINIMUM CLEAR COVER FOR
(a) SLAB = 15 MM
(b) BEAM = 25 MM
- PROVIDE 8 @ 300 C/C 1200 LONG BARS AS EXTRA TOP OVER THOSE BEAMS WHICH ARE PARALLEL TO MAIN REINFORCEMENT OF SLAB.
- BENT UP BARS SHALL BE EXTENDED UPTO 0.3 L IN ADJACENT SPAN OVER CONTINUOUS SUPPORT IF EXTRA TOPNOT PROVIDED AND ANCHOR DOWN IN END SUPPORT.
- NOT MORE THAN HALF THE COLUMN BARS SHALL BE LAPPED AT THE SECTION.
- USE OF NEEDLE VIBRATOR AND PLASTICIZERS RECOMMENDED FOR R.C.C. WORK.
- USE DENSIFIED COATED PLYWOOD FOR SHUTTERING AND FORMWORK.
- USE STEEL PROPS AND STEEL SUPPORT FOR FORMWORK.
- BURNT OIL NOT PERMITTED FOR DE-SHUTTERING.
- CURING - EXPOSED SURFACE OF CONCRETE SHALL BE KEPT CONTINUOUSLY IN WET CONDITION FOR MINIMUM 14 DAYS.
- CHODI OR CHAIR HEIGHT = SLAB THICKNESS - (15MM + DIA OF SLAB MAIN BAR).

KEY ELEVATION SHOWING NO. OF FLOORS CONSIDERED FOR STRUCTURAL DESIGN

SCHEDULE OF BEAMS AT SLAB LEVEL

BEAM NO.	SIZE/ THICKNESS BXD (OVERALL) M	MID SPAN MAIN BAR (MM)		TOP BARS (MM)	RINGS / DISTRIBUTION C/C (MM)
		BOTTOM STRAIGHT BARS	BOTTOM B/UP AT 2D FROM FACE		
B1	2 X 3	2 X 12 Φ	1 X 10 Φ	2 X 10 Φ	8 Φ @ 150 C/C
B2	2 X 4.5	2 X 12 Φ	2 X 10 Φ	2 X 10 Φ	8 Φ @ 150 C/C
B3	2 X 6	2 X 12 Φ	2 X 12 Φ	2 X 10 Φ	8 Φ @ 150 C/C



NOTES PADHO

Yaha Hoga Jugaad

*Thank
you!*

Website  www.notespadho.com

Facebook  @NotesPadho

Instagram  @NotesPadho